

GOLD CAMP SHOOTING SPORTS CENTER

Special Use Development Plan

CERTIFICATE OF OWNERSHIP

I, _____, HEREBY AFFIRM THAT I AM THE OWNER OR AUTHORIZED AGENT OF ALL INDIVIDUALS HAVING OWNERSHIP OR INTEREST IN THE PROPERTY DESCRIBED HEREIN, KNOWN AS _____, TELLER COUNTY SHOOTING SOCIETY, AND ACKNOWLEDGE THE ZONING AND CONDITIONS AS APPLICABLE TO THIS PROPERTY AND DEVELOPMENT.

THE FOREGOING WAS ACKNOWLEDGED BY ME THIS _____ DAY OF _____, 20____, AD
BY (NAME) _____, AS (TITLE) _____,
OF _____, AN AUTHORIZED SIGNATORY.

OWNER OF RECORD:

WITNESS MY HAND AND OFFICIAL SEAL:

MY COMMISSION EXPIRES

NOTARY PUBLIC:

STATE OF COLORADO)

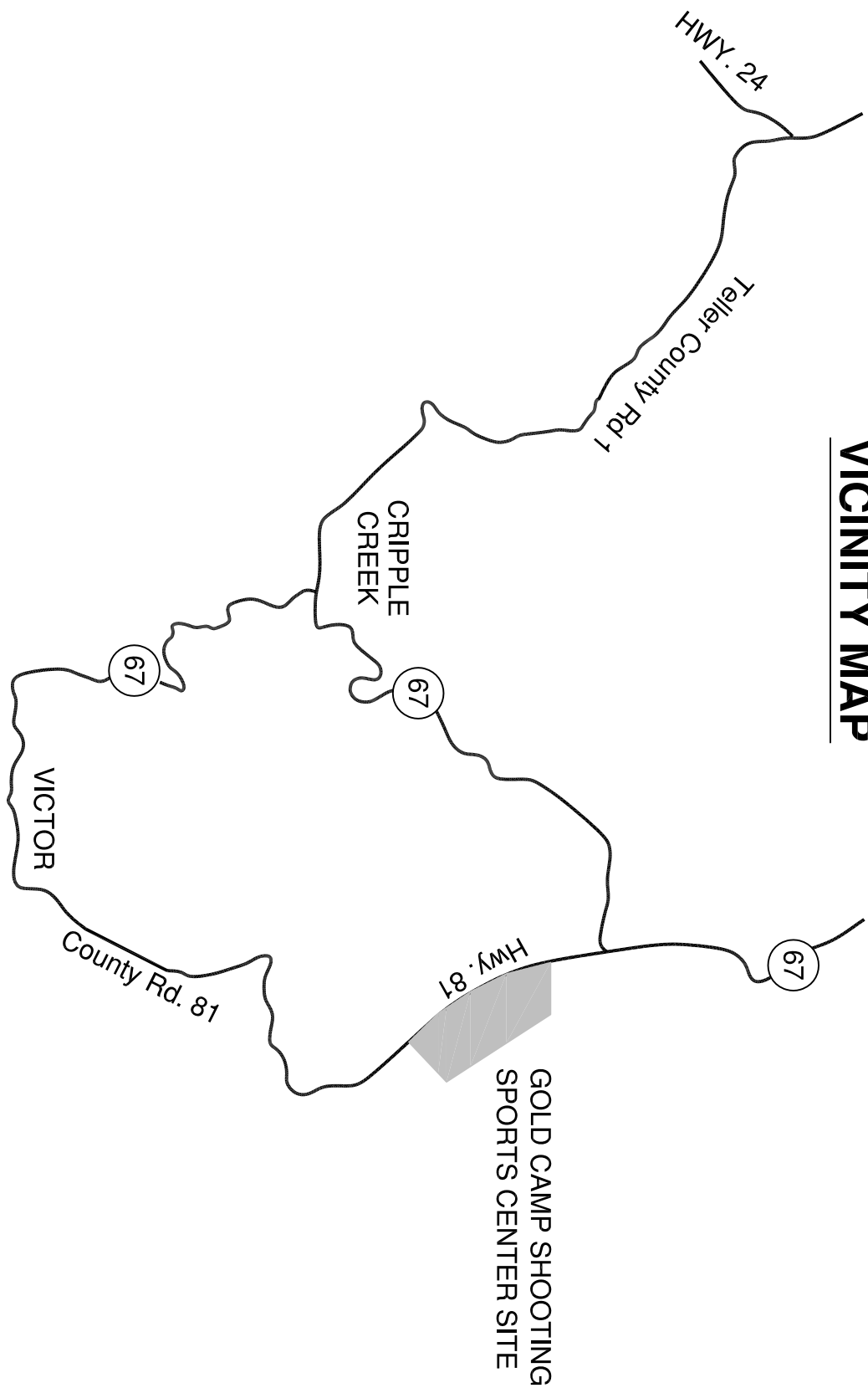
COUNTY OF TELLER)

COUNTY APPROVAL

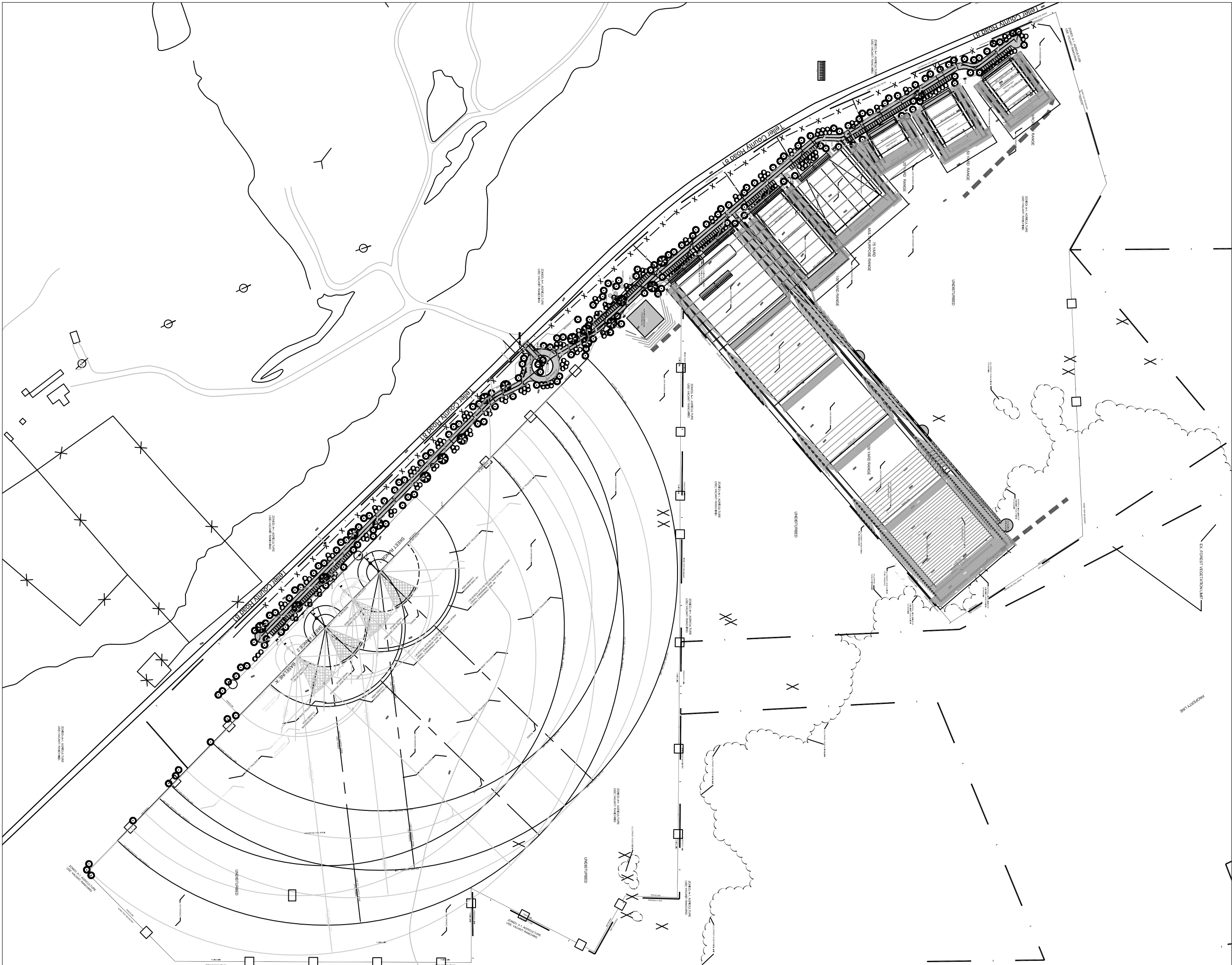
APPROVAL IS GRANTED THIS DAY OF 20 AT

PLANNING DIRECTOR

VICINITY MAP



Sheet Key:



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Owners:

CRIPPLE CREEK & VICTOR GOLD MINING COMPANY
Attn: Raymond Dubois, VP & General Manager
P.O. Box 191- 100 North 3rd Street
Victor, CO 80860
Phone: 719-689-4044

Owner's Rep/Applicant:

TELLER COUNTY SHOOTING SOCIETY (TCSS)
Attn: Craig Chellis, President, Drew Ganter, Project Manager
P.O. Box 548
Woodland Park, CO 80863
Phone: 719-687-8677

Planner/Landscape Architect

WILLIAM GUMAN & ASSOCIATES
Bill Guman, RLA, Jason Alwine, RLA
731 N. Weber St., Suite 10
Colorado Springs, CO 80903
Phone: 719-633-9700

Engineers (Civil)

Mike Bartusek, P.E., Jason Helton, P.E.
3520 Austin Bluffs Pkwy., Suite 200
Colorado Springs, Colorado 80918
Phone: 719-266-5212

Engineers (Geotechnical) Architectural (Structural)

Mark Weidhass, P.E., Nate Dowden, P.E.
2910 Austin Bluffs Pkwy
Colorado Springs, Colorado 80918
Phone: 719-548-0600

NOTE: ALL DRAWINGS ARE REPRESENTATIVE OF THE EXISTING CONDITIONS. ANY CHANGES WHICH ARE MADE WITHOUT PRIOR APPROVAL OF ENTITIES HAVING JURISDICTION OVER THE PROJECT MAY RESULT IN DELAY OF FINAL ACCEPTANCE AND BE THE RESPONSIBILITY OF THE OWNER. THE CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING ALL NECESSARY PERMITS AND APPROVALS FROM THE APPROPRIATE AGENCIES.

 **William Guman**
& Associates, Ltd.

EST. 1982
URBAN PLANNING | COMMUNITY DESIGN | LANDSCAPE ARCHITECTURE

731 North Weber Street, Suite 10
Colorado Springs, CO 80903
719.633.9700 fax 719.633.4250
Email: WGuman@aol.com

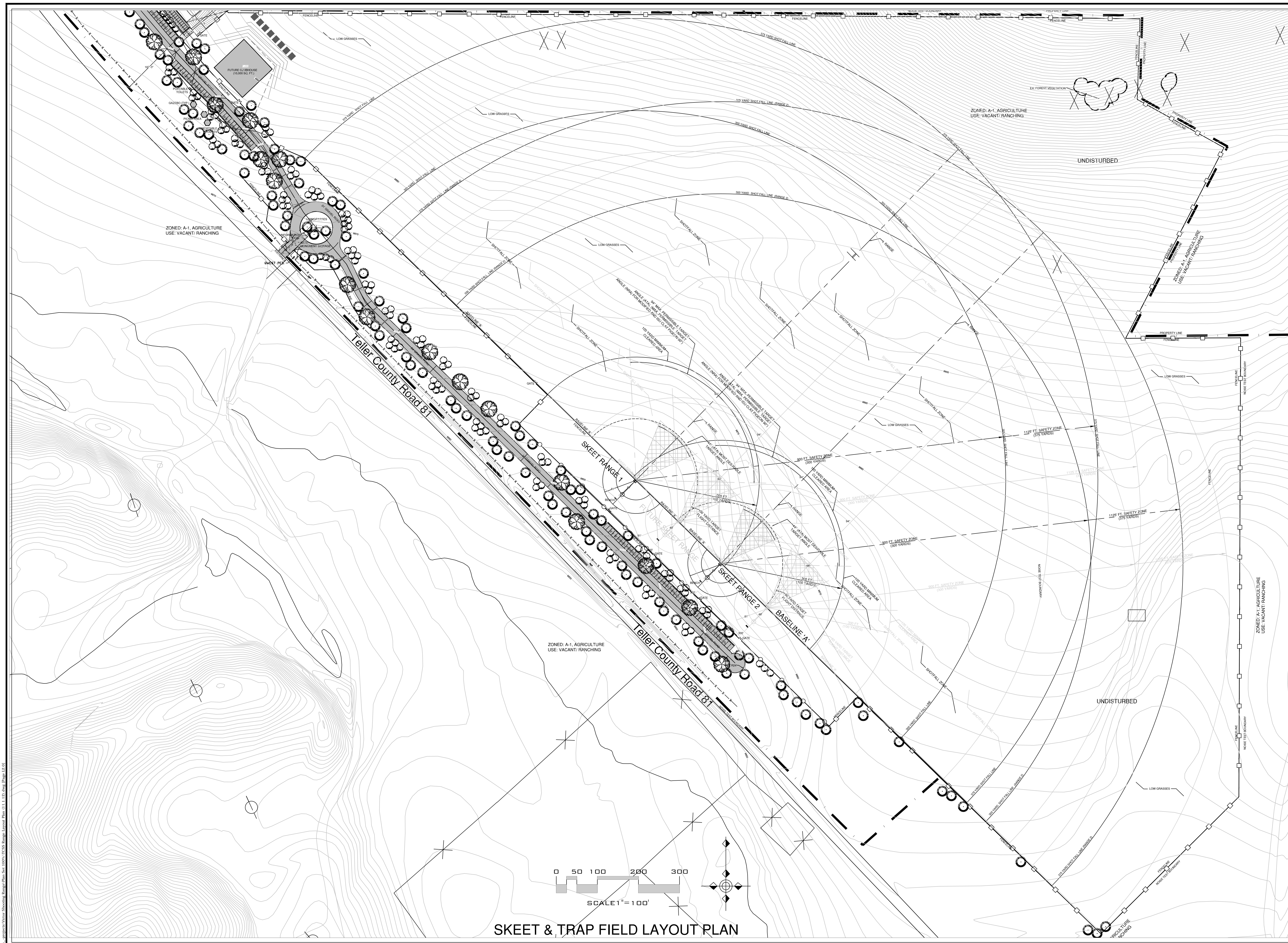
MEMBERS AMERICAN SOCIETY OF LANDSCAPE ARCHITECTS

Gold Camp Shooting Sports Center
County Road 81
TELLER COUNTY, CO

DATE: 11/5/2012		
DEMN: WFG		
CHECKED: JRA		
REVISIONS:		
DATE: BY: COMMENTS:		

COVER
PAGE

1
OF 16 SHEETS



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Gold Camp Shooting Sports Center
County Road 81
TELLER COUNTY, CO

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[illegible]

SHEET & TRAP FIELD PLAN	
SHEET NO.	2 OF 16 SHEETS

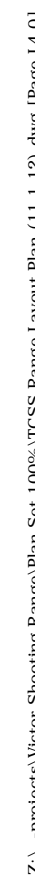


Diagram illustrating the components and dimensions of a 4' x 4' wire mesh fence section:

- END OR CORNER POST - WOOD**: 8" dia. (top) X 7'6" length
- LINE POST - WOOD**: 4" dia. (top) X 7'0" length
- 4" x 4" WOOD BRACE**: LOOP OF 9 GA. WIRE TWISTED TO TIGHTEN
- BARBED WIRE**: Indicated by curved arrows pointing to the top and bottom horizontal wires.
- GRADE**: Indicated by a horizontal line with diagonal hatching below the fence section.
- OPT. POST - STEEL**: 6'6" length (shown as an alternative post type on the right).
- Dimensions**: The fence section is 4' wide and 4' high. The vertical spacing between the horizontal wires is 12" (indicated by four 12" segments on the right side).

END OR CORNER POST - WOOD
8" dia. (top) X 7'6" length

CHAIN AND PADLOCK

MIN. 1-5/8 TUBULAR STEEL

STANDARD HINGE

END OR CORNER POST - WOOD
8" dia. (top) X 7'6" length

4" x 4" WOOD BRACE

BARBED WIRE

STAPLE

GRADE

GRAVEL SURFACE

GATE OPENING (Per Plan)

END PANEL

PANEL BRACE

12"

12"

12"

12"

BARBED WIRE APRON ON EXTENSION ARMS

1'0"

TIE WIRES OR HOG PINS

TOP RAIL OR TENSION WIRE

BRACE RAIL

TRUSS ROD

3/8" MIN. DIA.

CORNER, END, OR PULL POST

2"

BOTTOM OF MESH

GRADE

3/6" MIN.

10" MIN. DIA.

HOG RINGS (Typ.)

TENSION WIRE

6'0"

10'0" MAX.

10'0" MAX.

LINE POSTS TO BE EQUALLY SPACED

LINE POST

3/8" PLAIN PIN (Riveted flush)

LOCK PIN (Typ.)

1'0"

3/8" PLAIN PIN (Riveted flush)

CORNER POST

EXTENSION ARM DETAILS

CHAIN LINK SECURITY FENCE (SAFETY AREAS AND STRUCTURE ENTRANCES)

NTS

William Guman
& Associates, Ltd.
Landscape Architecture
Urban Planning Community Design



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Colorado Springs, CO 80903
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Email: WGuman@aol.com

MEMBER AMERICAN SOCIETY OF LANDSCAPE ARCHITECTS

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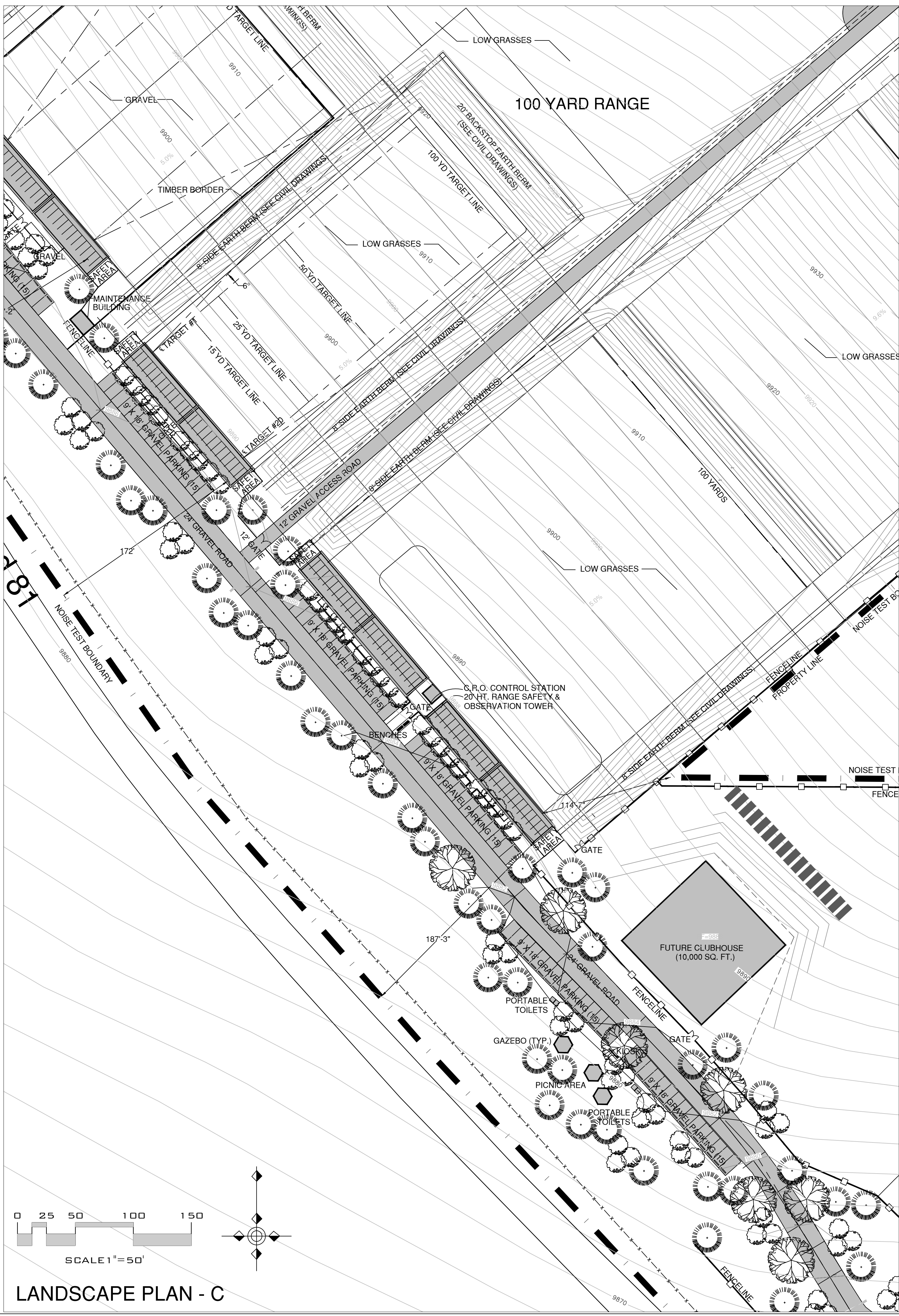
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ARCHERY AND
15 TO 100 YARD
PISTOL RANGE

SHEET NO.

4

OF 16 SHEETS



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Gold Camp Shooting Sports Center

County Road 81

TELLER COUNTY, CO

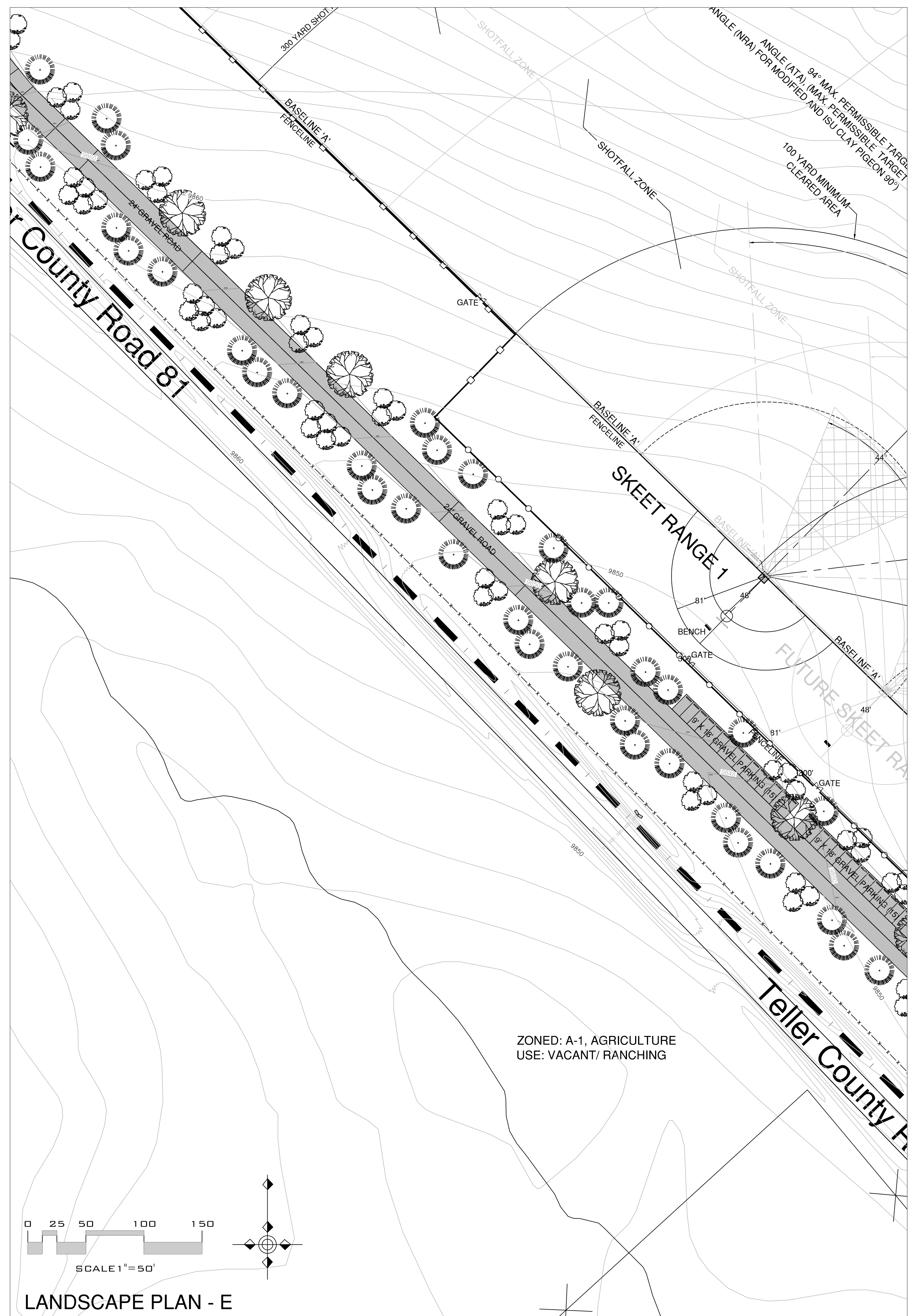
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REVISIONS:		
DATE:	BY:	COMMENTS:

REVISIONS:		
DATE:	BY:	COMMENTS:

ARCHERY AND
15 TO 100 YARD
PISTOL RANGE

SHEET NO.
5
OF 16 SHEETS



Gold Camp Shooting Sports Center
County Road 81
TELLER COUNTY, CO

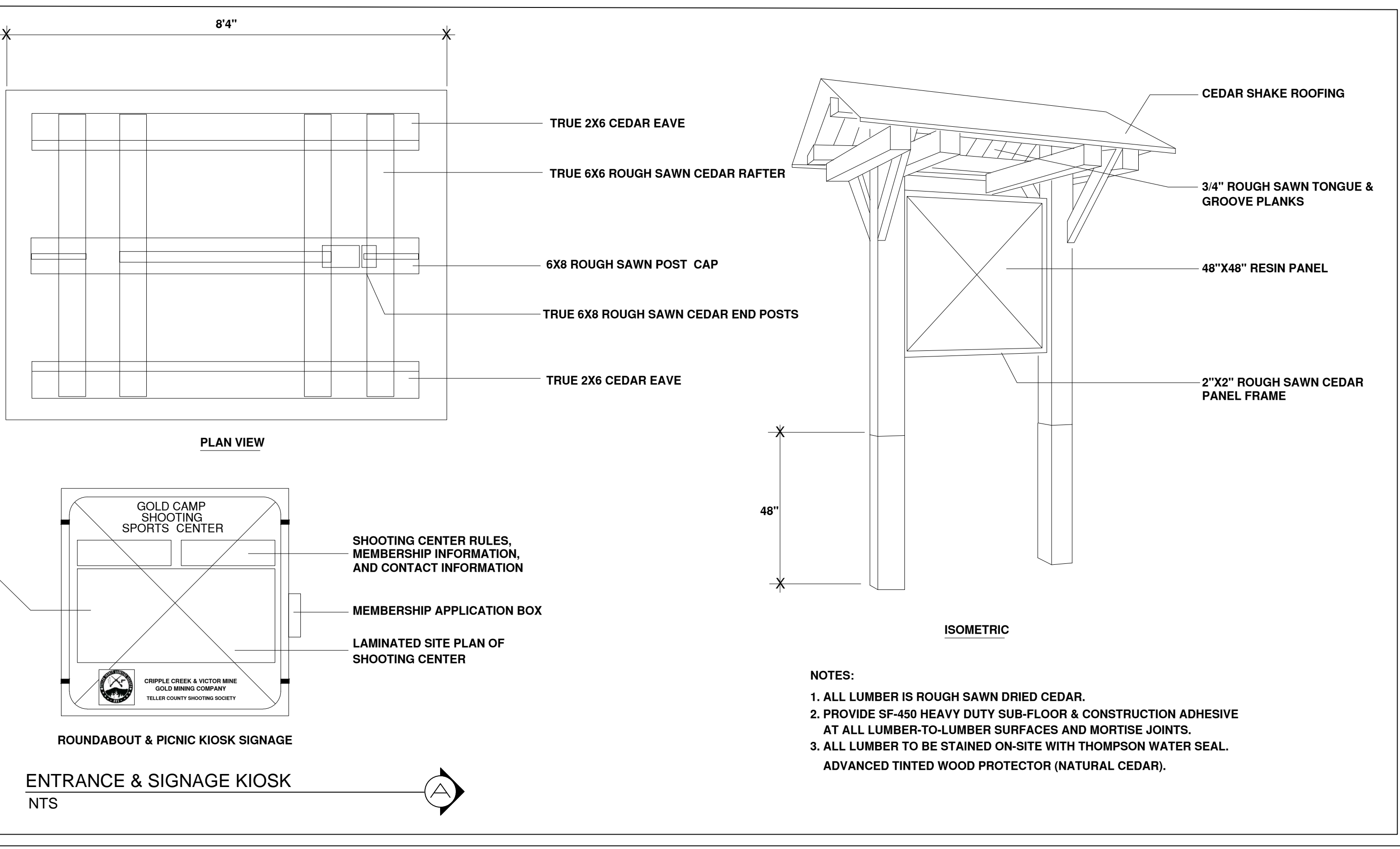
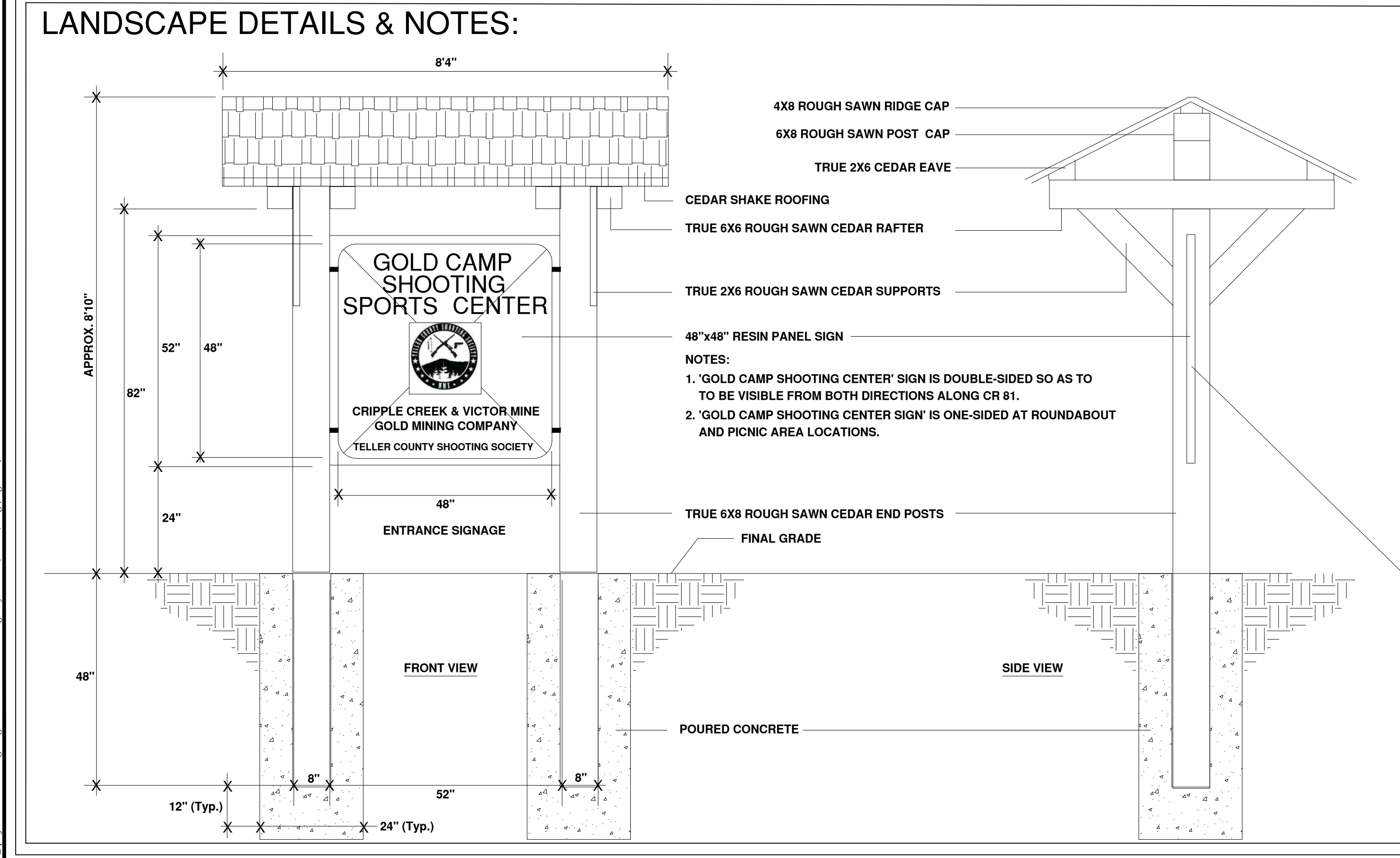
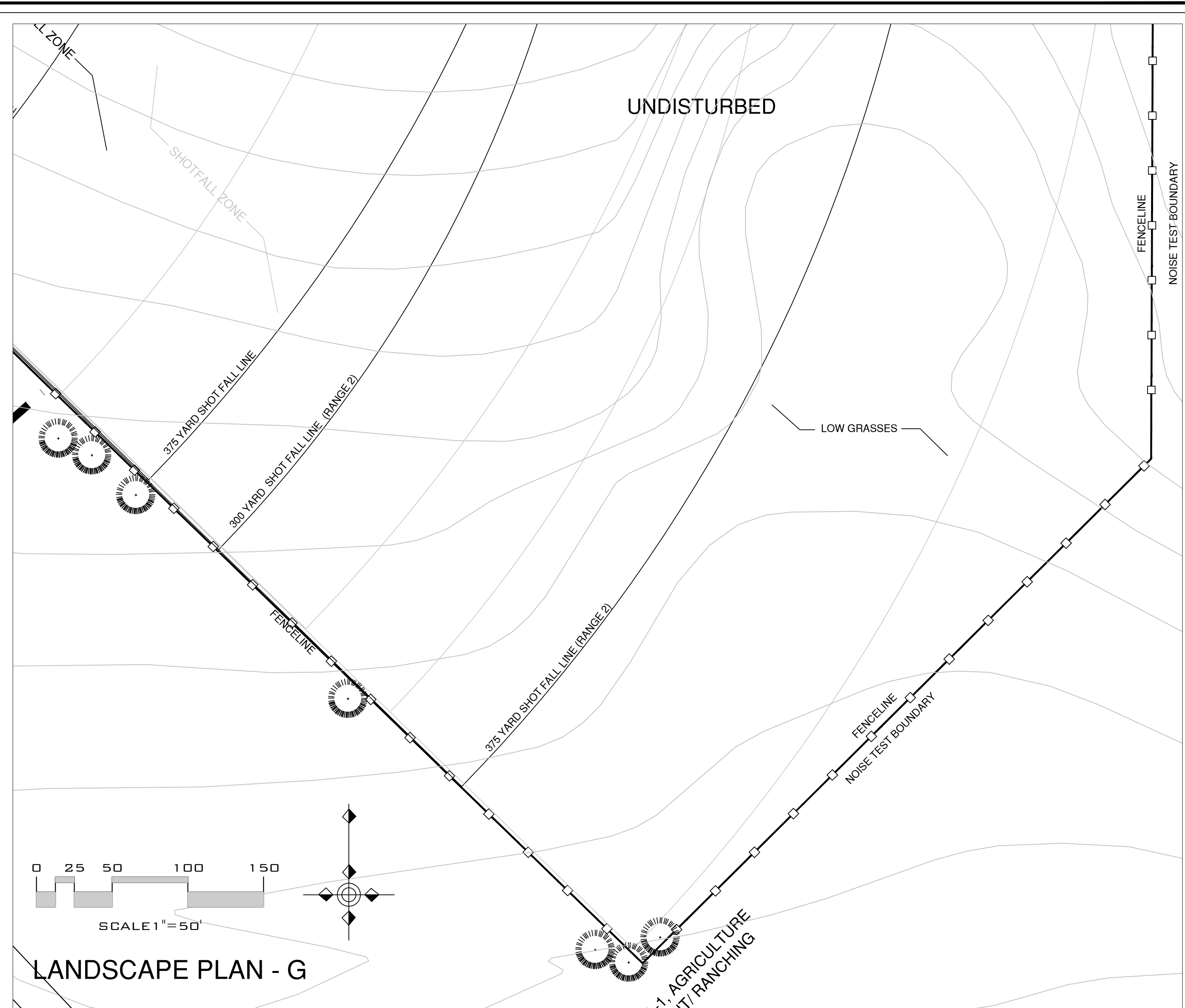
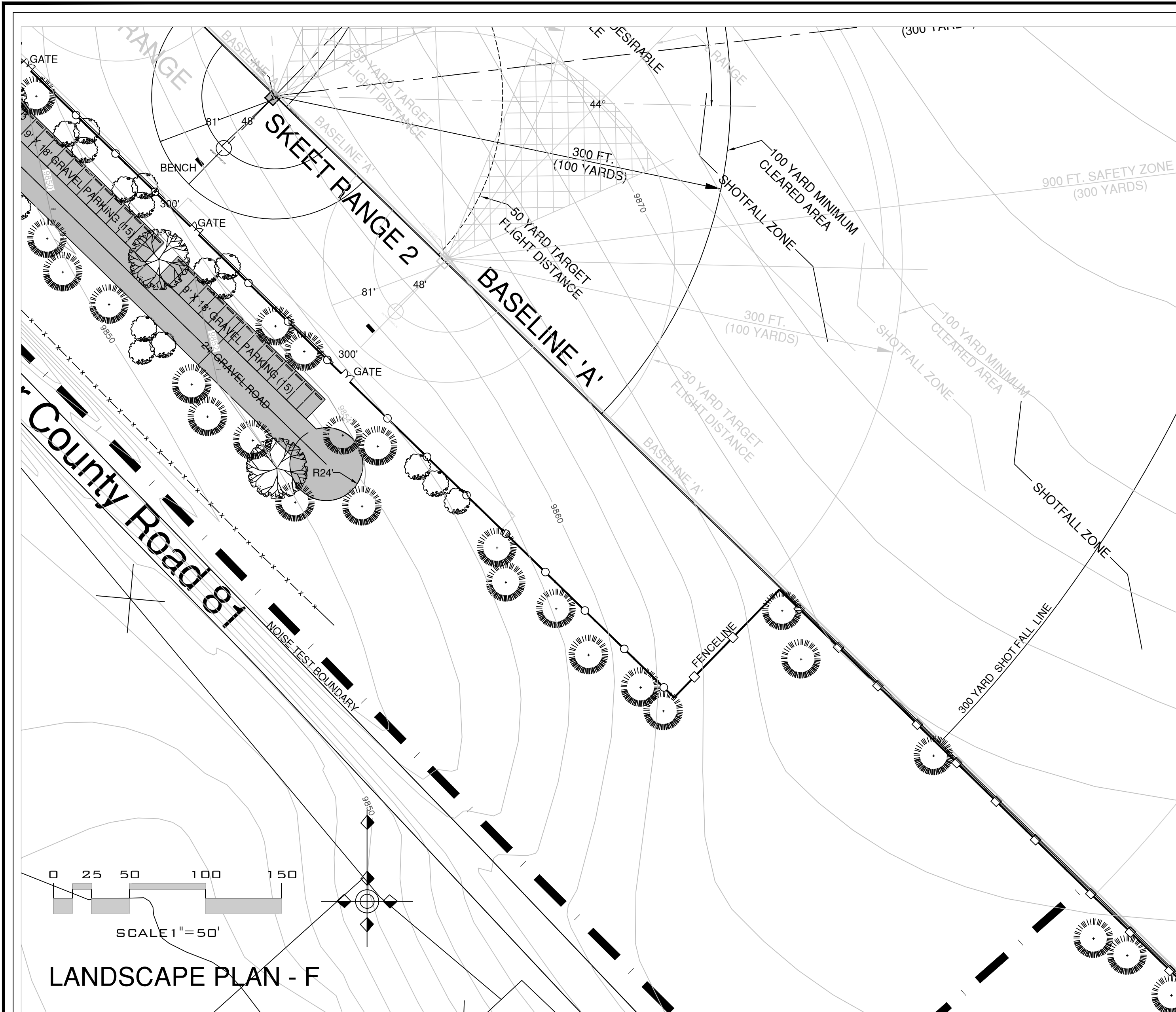
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ARCHERY AND
15 TO 100 YARD
PISTOL RANGE

SHEET NO.

6
OF 16 SHEETS



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MEMBER AMERICAN SOCIETY OF LANDSCAPE ARCHITECTS

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Gold Camp Shooting Sports Center

County Road 81

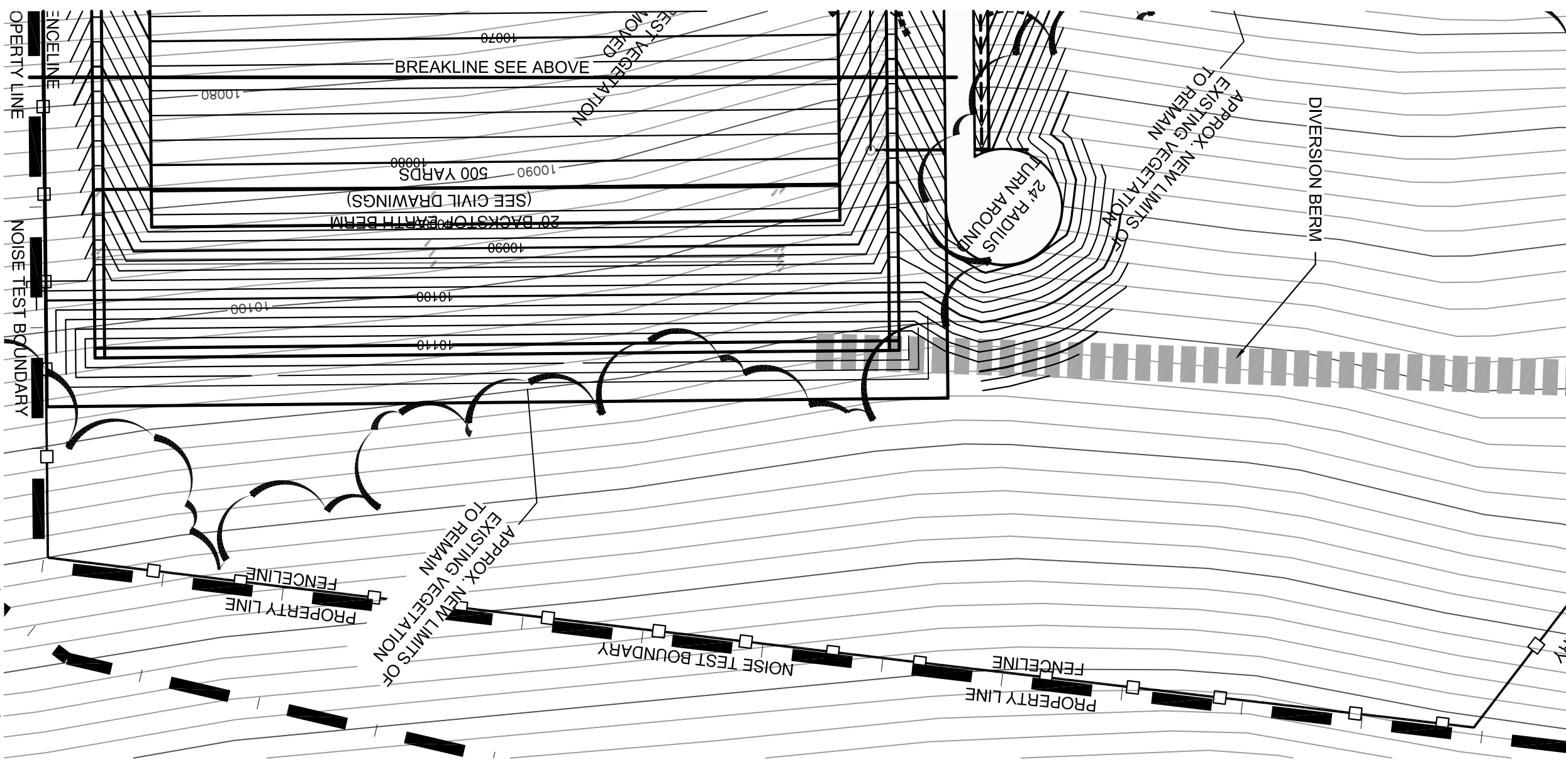
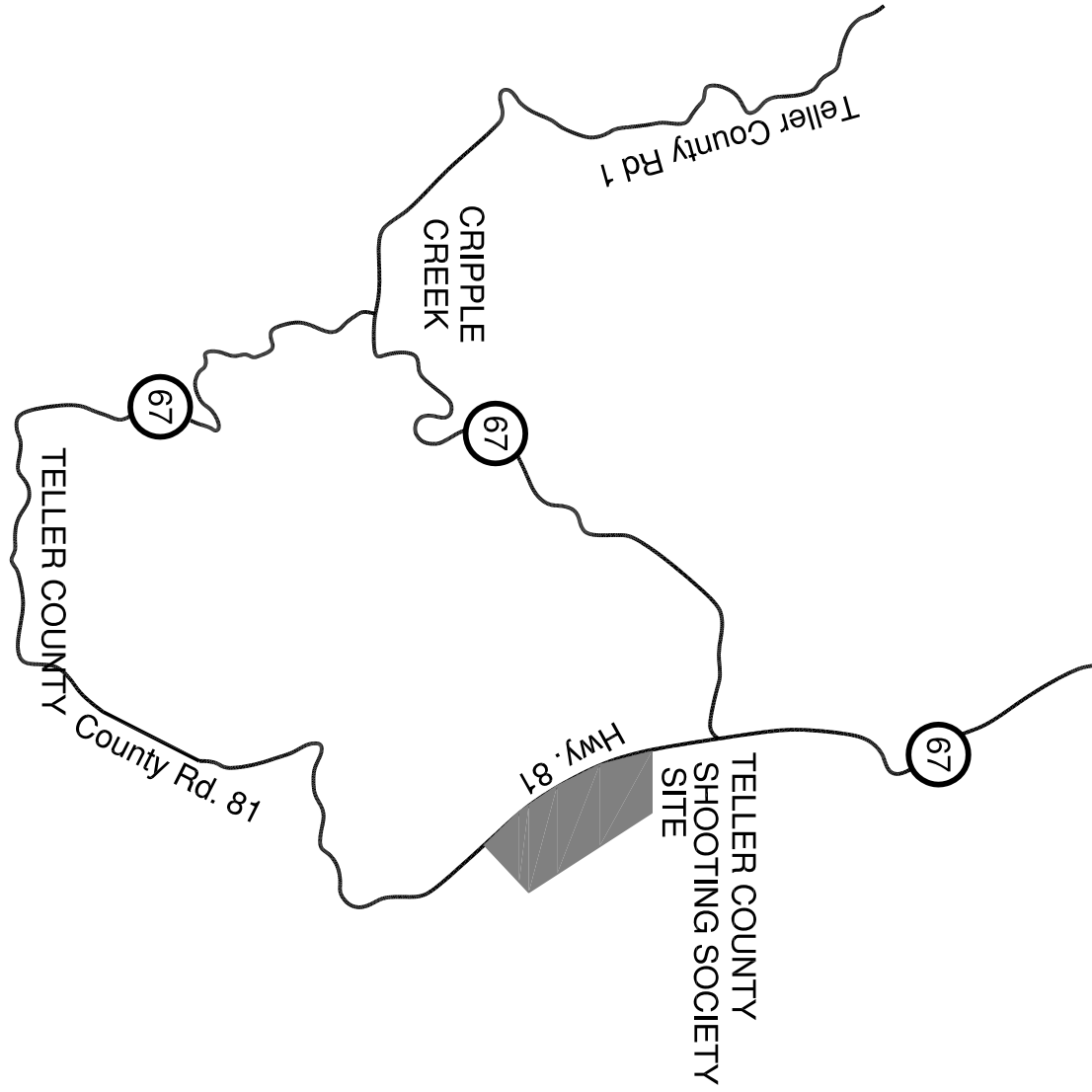
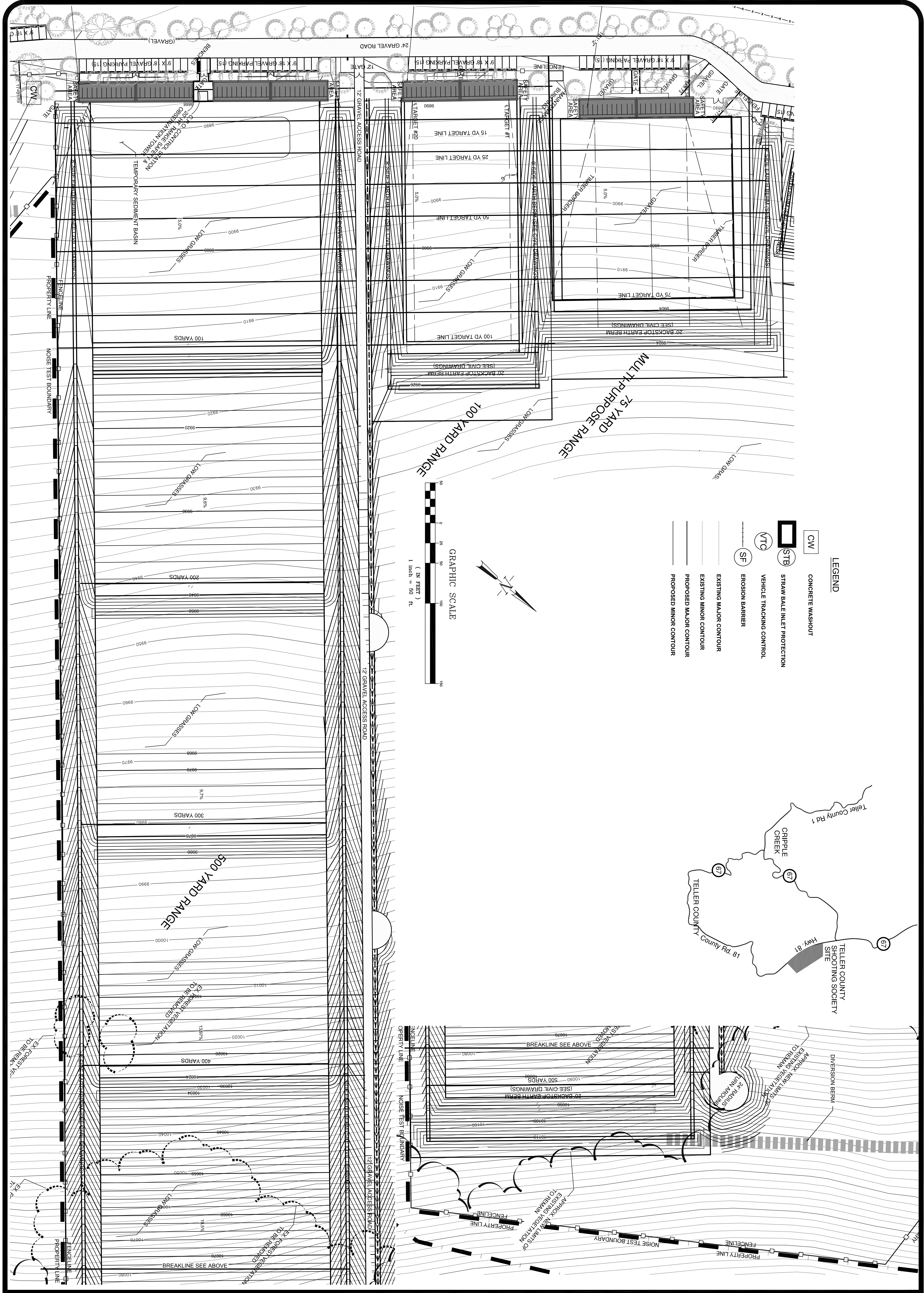
TELLER COUNTY, CO

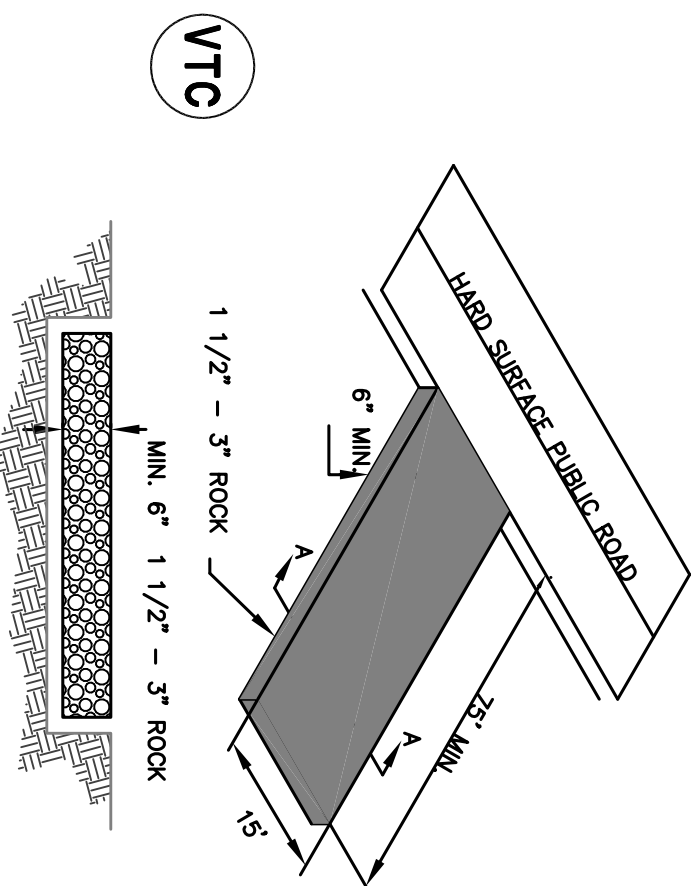
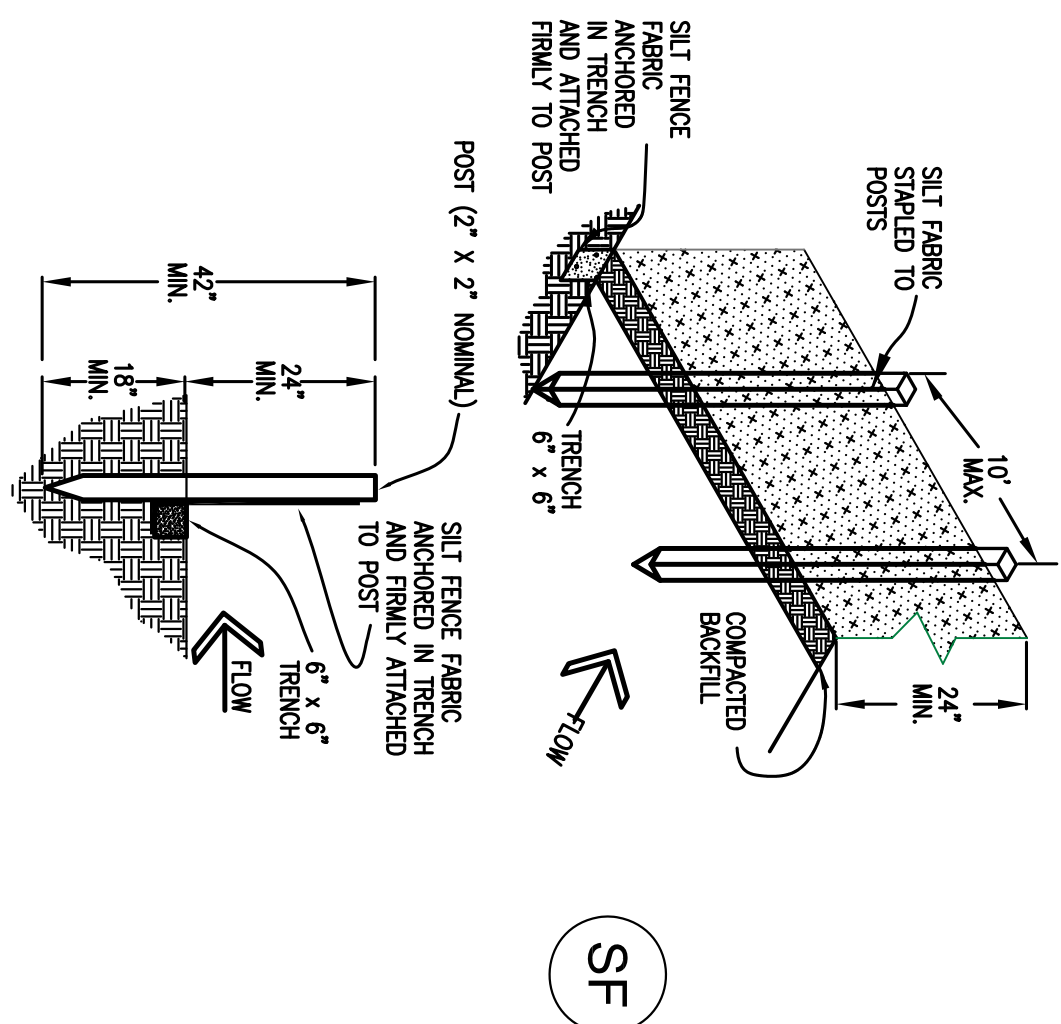
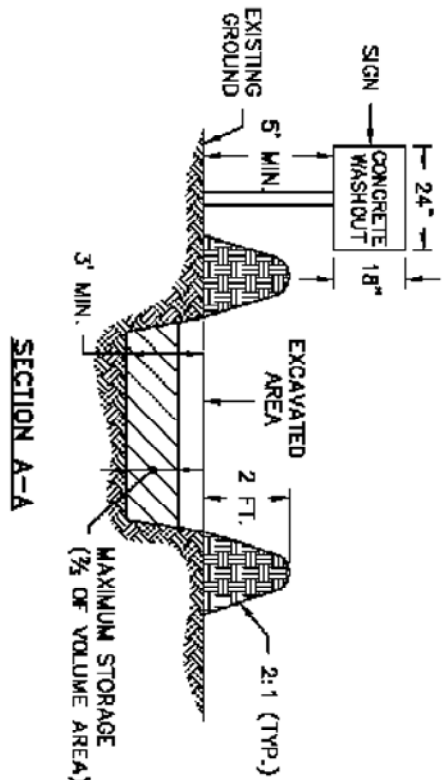
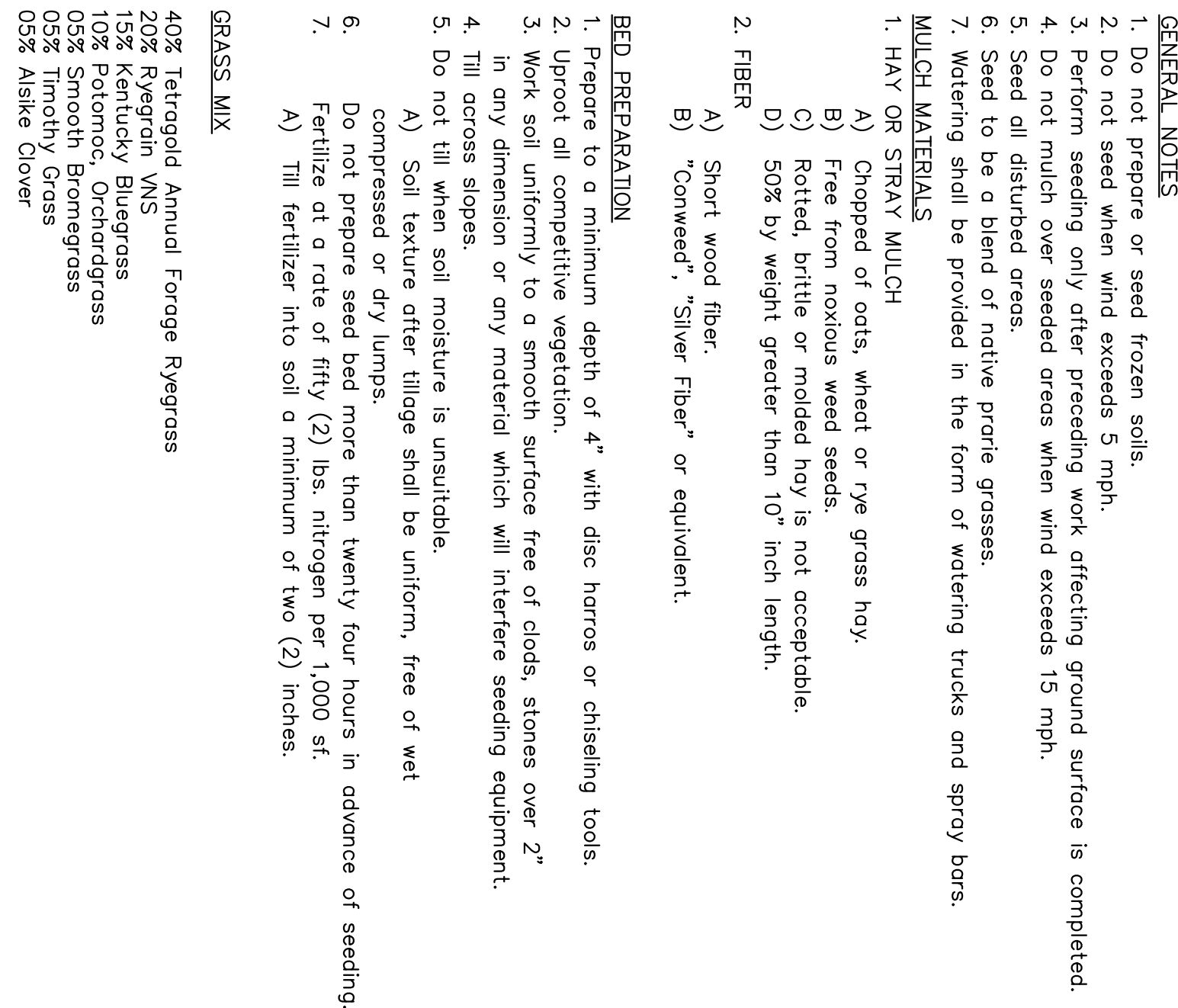
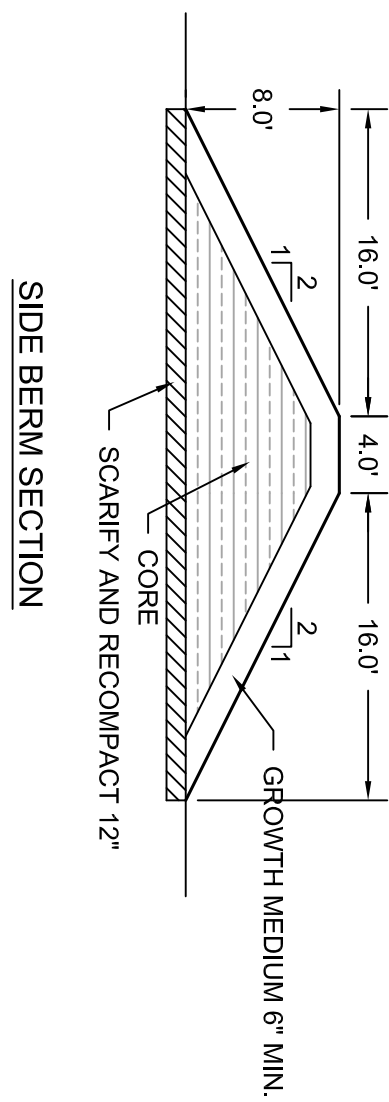
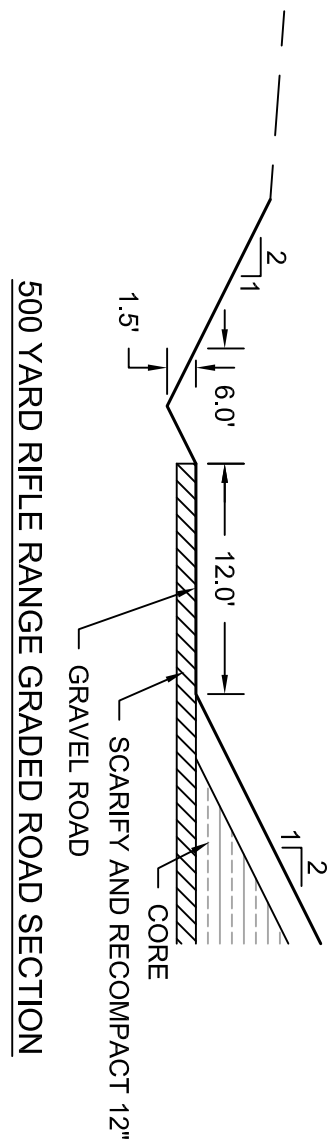
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DRAWN: WFG
CHECKED: JRA

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DATE:	BY:	COMMENTS:

ARCHERY AND 15 TO 100 YARD PISTOL RANGE

SHEET NO. **7** OF 16 SHEETS





SECTION A-A

N.T.S.

SILT FENCE NOTES

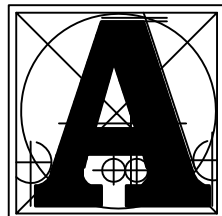
INSTALLATION REQUIREMENTS

INSTALLATION REQUIREMENTS

1. SILT FENCES SHALL BE INSTALLED PRIOR TO ANY LAND DISTURBING ACTIVITIES.
 2. WHEN JOINTS ARE NECESSARY, SILT FENCE GEOTEXTILE SHALL BE SPUNTED TOGETHER AT SUPPORT POST AND SECURELY SEALED.
 3. METAL POSTS SHALL BE STUDDED TEEL OR T-YE WITH MINIMUM WEIGHT OF 13.3 POUNDS PER LINEAR FOOT. SECOND POSTS SHALL HAVE A MINIMUM DIAMETER OR CROSS SECTIONAL DIMENSION OF 2 INCHES.
 4. THE FILTER MATERIAL SHALL BE FASTENED SECURELY TO THE TOP OF EACH WOOD POST USING TWO NAILS. FOR WOOD POSTS, THE GEOTEXTILE SHALL NOT BE STAPLED TO EXISTING TREES.
 5. WHILE NOT REQUIRED, WIRE MESH FENCING MAY BE USED TO FASTENER THE GEOTEXTILE TO THE UPLOUSE SIDE OF THE TRENCH. THE WIRE SHALL BE NO LONGER THAN 6 FEET LONG. THE WIRE SHALL NOT EXTEND MORE THAN 3' ABOVE THE ORIGINAL GROUND SURFACE.
- A. ALONG THE LEVEL OF CUT, INSTALL THE SILT FENCE ALONG A LEVEL CONTOUR AND PROVIDE AN OVERLAP OF 6 INCHES BETWEEN SECTIONS AND SEDIMENT TO SETTLE A MINIMUM DISTANCE OF 5 FEET FROM THE JOB OF THE SILT IS RECOMENDED.
- B. THE HEIGHT OF THE SILT FENCE FROM THE GROUND SURFACE SHALL BE MINIMUM OF 24 INCHES AND SHALL NOT EXCEED 36 INCHES; HIGHER FENCES MAY IMPOUND VOLUMES OF WATER SUFFICIENT TO CAUSE FAILURE OF THE EARTH DAM.
- ### MANTENANCE REQUIREMENTS
1. MAINTAIN THE SILT FENCE THROUGHOUT ALL PERIODS OF RAINFALL. AFTER EACH RAINFALL EVENT, CHECK AT LEAST DAILY DURING PROLONGED RAINFALL AND REMOVE ANY DEBRIS, LOGS, LIMBS, BRANCHES, UNWANTED ROOTS, COULD PASS, UNSTRENGTHENED OR DAMAGED SILL FENCES SHALL BE PROMPTLY REPAIRED OR REPLACED.
2. SEGMENT SHALL BE REMOVED FROM BEHIND THE TRENCH IMMEDIATELY AFTER IT HAS BEEN PLACED TO HALP THE EXPOSED GEOTECHNICAL HEATH REPAIR.
3. SILT FENCES SHALL BE REMOVED WHEN APPROPRIATE RESERVATIVE COVER IS ATTAINED AS APPROVED BY THE CITY.

[illegible]

3520 Austin Bluffs Parkway
Suite 200
Colorado Springs, CO 80918
(719) 266-5212
fax: (719) 266-5341



Associated Design Professionals, Inc

DATE:
11/2/12

JOB NO.
111006

CAD FILE NO
BASE.DWG

DRAWN BY
JRH

DESIGNED BY
JRH

PROJECT ENGINEER
MAB

PROJECT MANAGER
MAB

SCALE:
HORZ. 1" = 100'

GOLD CAMP SHOOTING SPORTS CENTER
COUNTY ROAD 81
TELLER COUNTY, COLORADO

GRADING/EROSION CONTROL DETAILS



Corporate Office
2910 Austin Bluffs Parkway
Colo. Spgs., CO 80918
Voice (719) 548-0600
Fax (719) 548-0223
Castle Rock Office:
(303) 688-9475
Woodland Park Office:
(719) 687-6077
Monument Office:
(719) 488-2145
Pueblo Office:
(719) 544-7750



**GOLD CAMP SHOOTING
SPORTS CENTER**
CR 81
TELLER COUNTY, COLORADO
TELLER COUNTY SHOOTING SOCIETY

ENGINEER:	MW
DRAWN BY:	CL
CHECKED BY:	MW
PRINTING:	DATE:
SUBMITTAL	11-05-12

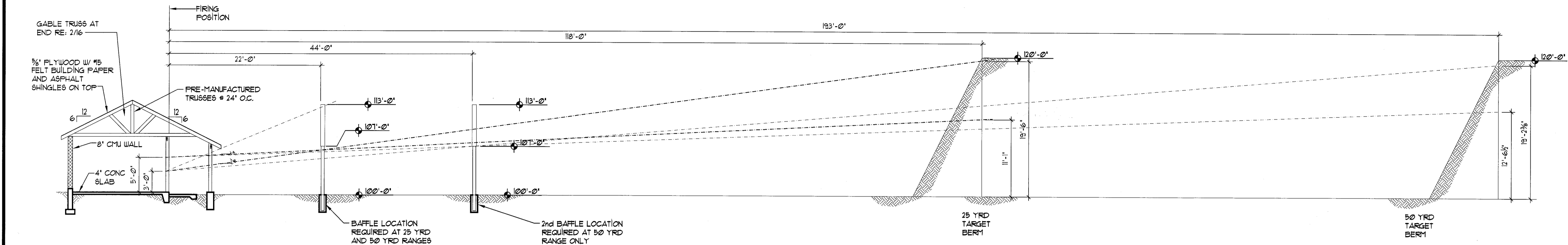
REVISION: DATE:

FOUNDATION FRAMING PLANS SECTIONS AND DETAILS

SHEET No.

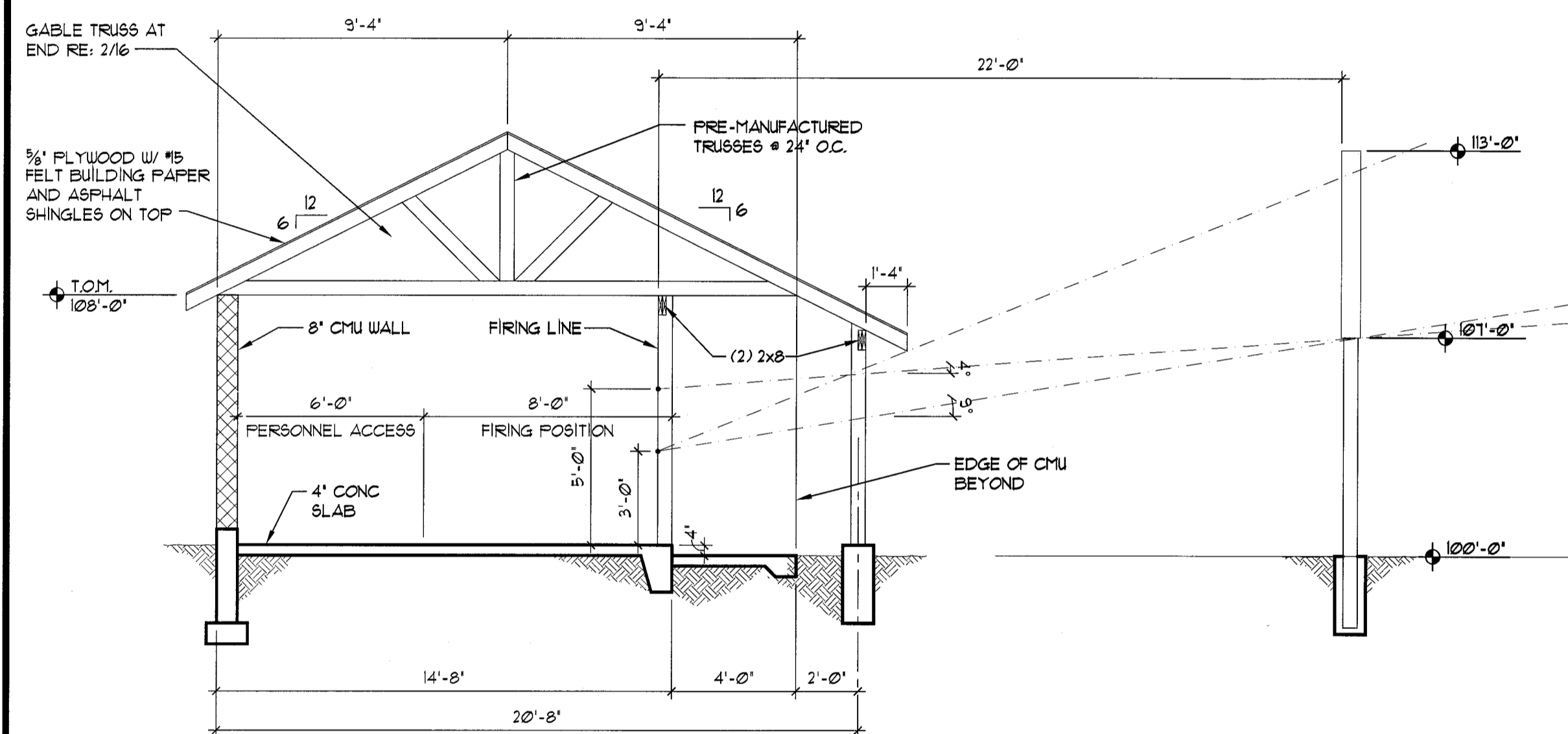
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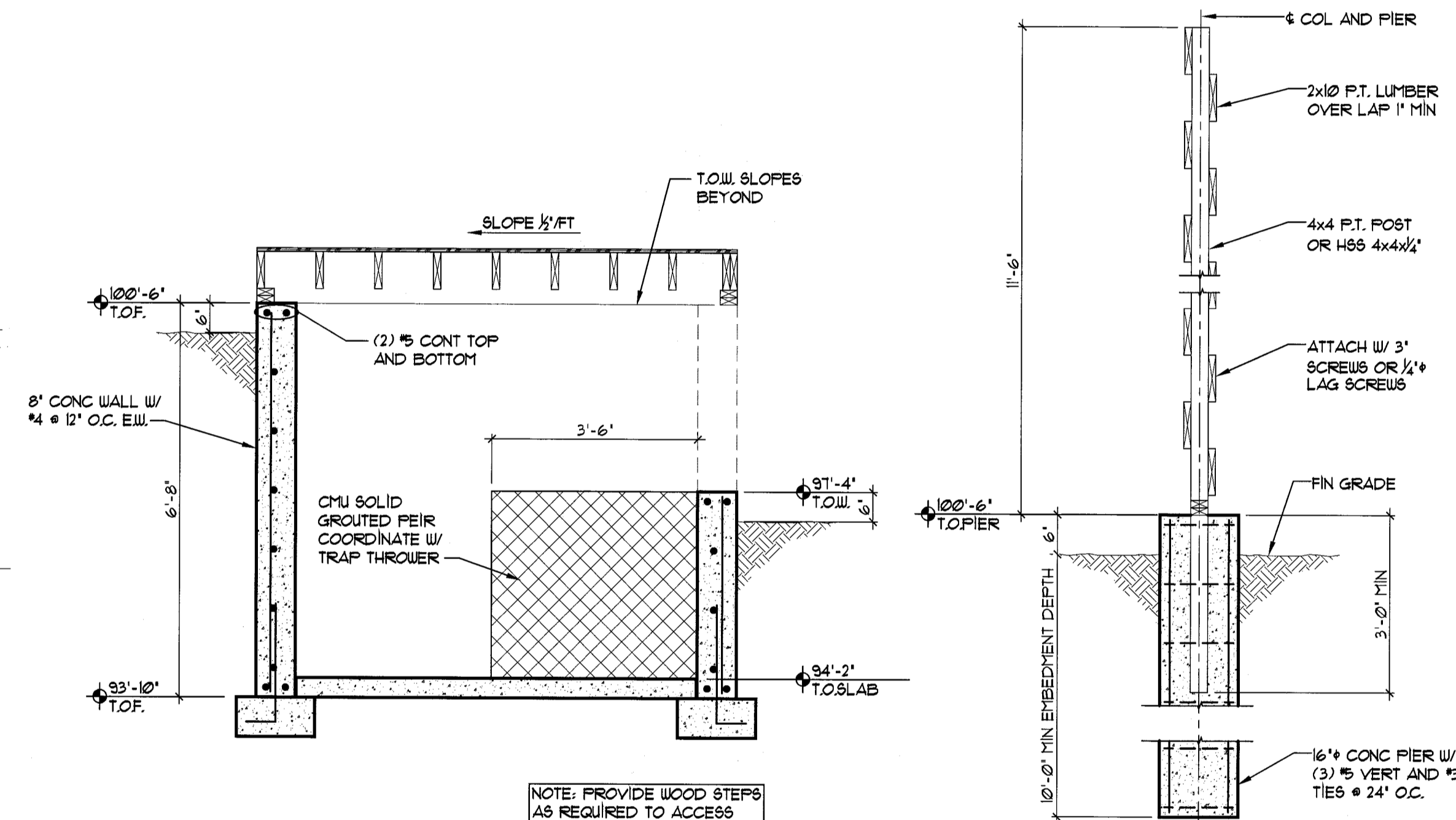
RANGE SECTION

— SCALE: 1/8" = 1'-0"



FIRING LINE STRUCTURE SECTION

— SCALE: 1/4" = 1'-0"



1 TRAP HOUSE SECTION
14 SCALE: 1/2" = 1'-0"

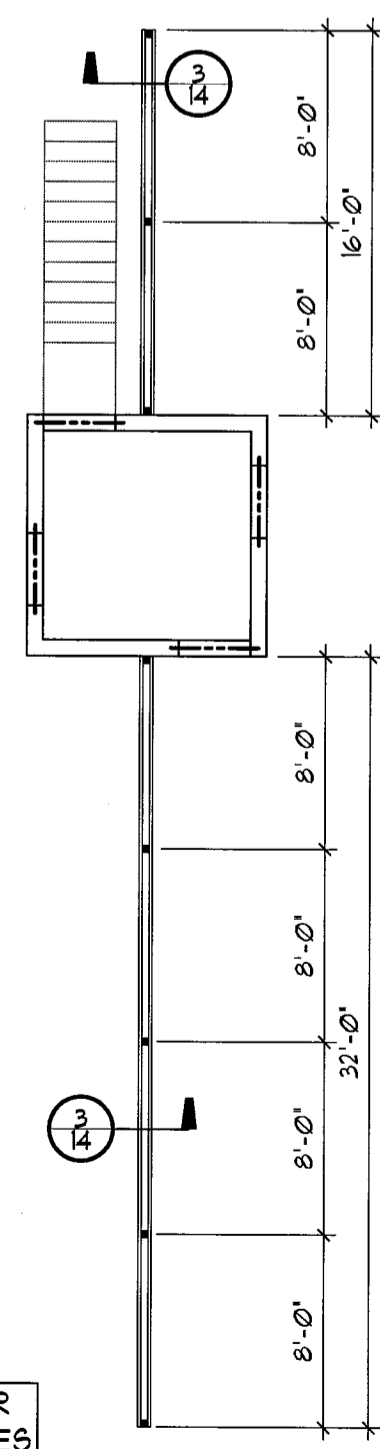
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3
14

SKEET HOUSE WALL
FOUNDATION DETAIL

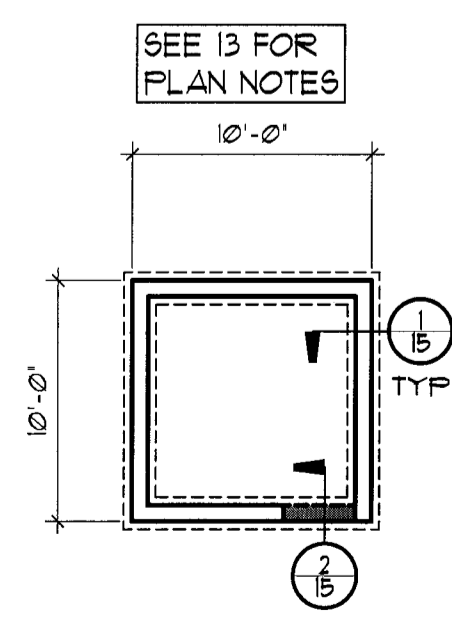
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SCALE: 1/2" = 1'-0"



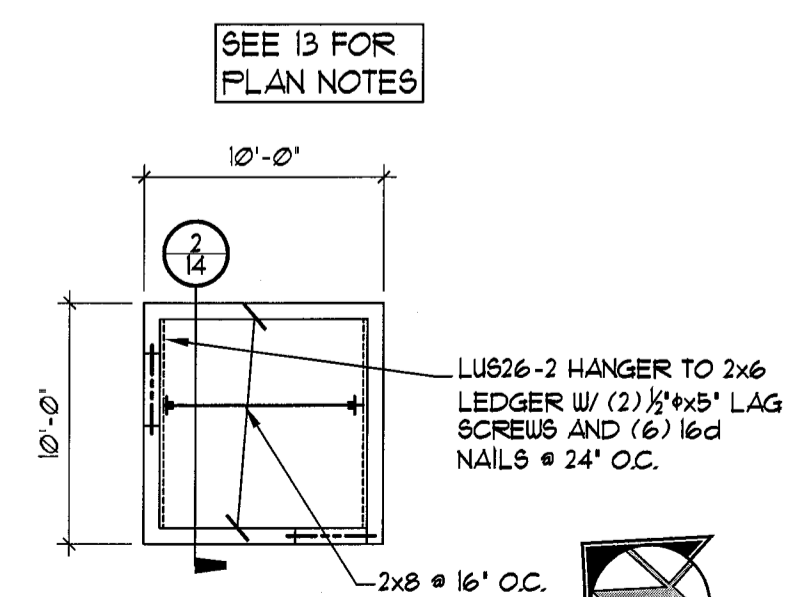
NET HOUSE WALL PLAN VIEW

- SCALE: 1/8" = 1'-0" -



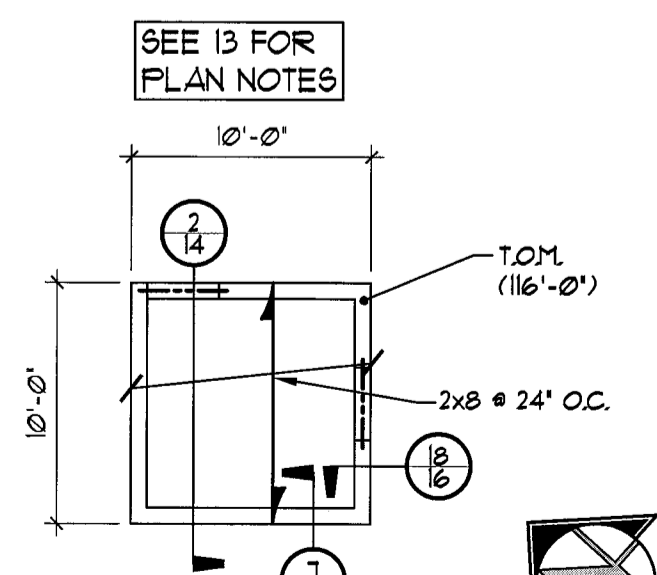
**SKEET HOUSE
FOUNDATION PLAN**  NORTH

— SCALE: 1/8" = 1'-0"



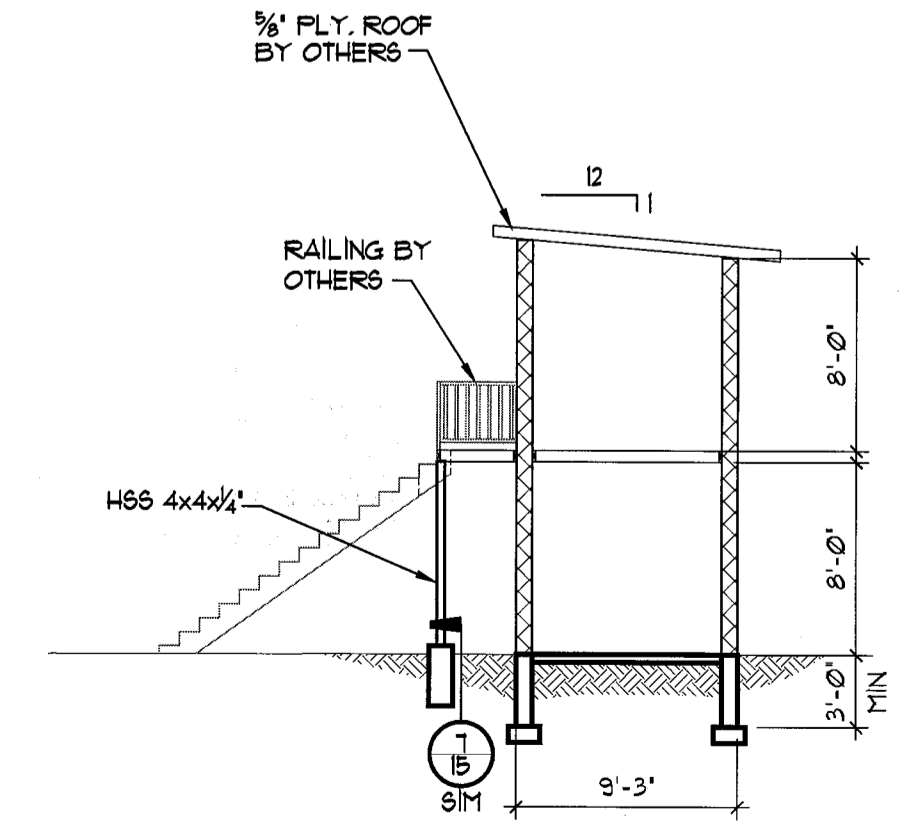
SKEET HOUSE
FLOOR FRAMING PLAN

— SCALE: 1/8" = 1'-0" —



SKEET HOUSE
ROOF FRAMING PLAN

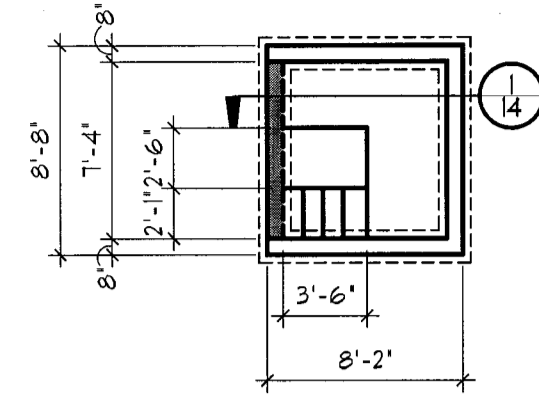
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2 HI-LOW SKEET HOUSE SECTION
14
SCALE: 1/8" = 1'-0"

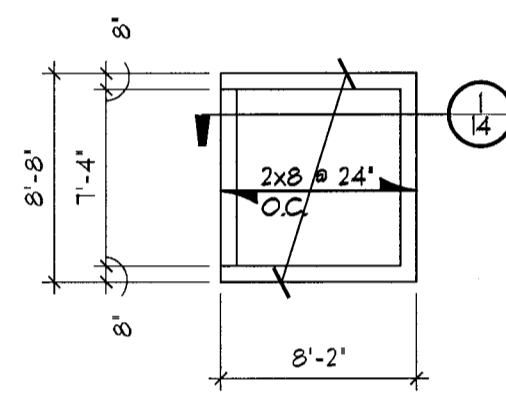
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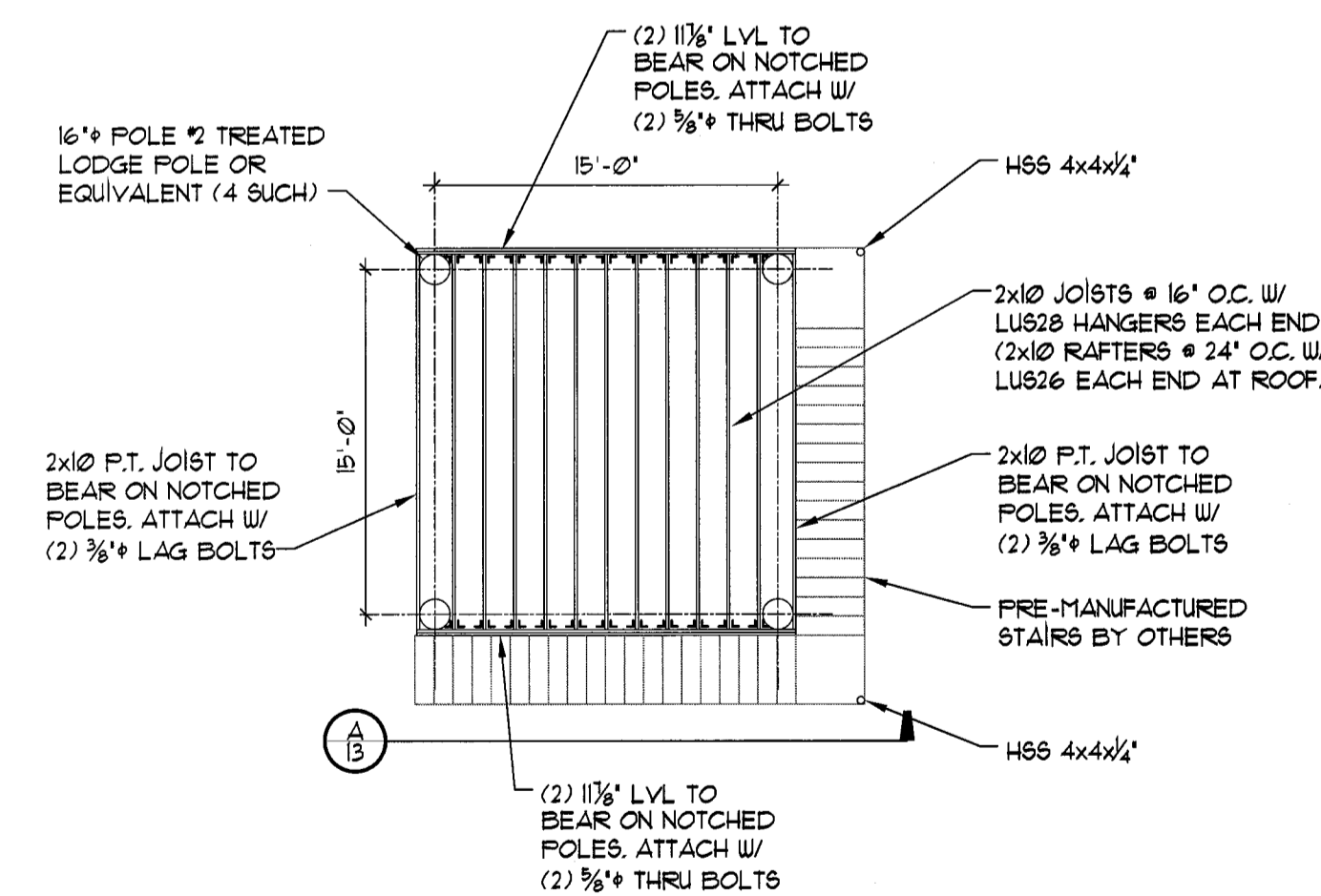
TRAP HOUSE FOUNDATION PLAN NORTH

SCALE: 1/8" = 1'-0"



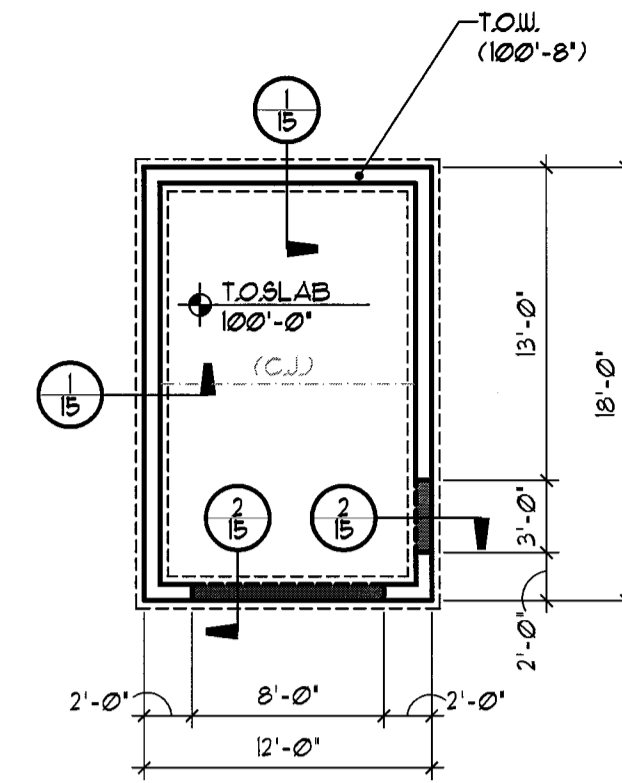
TRAP HOUSE ROOF FRAMING PLAN NORTH

SCALE: 1/8" = 1'-0"



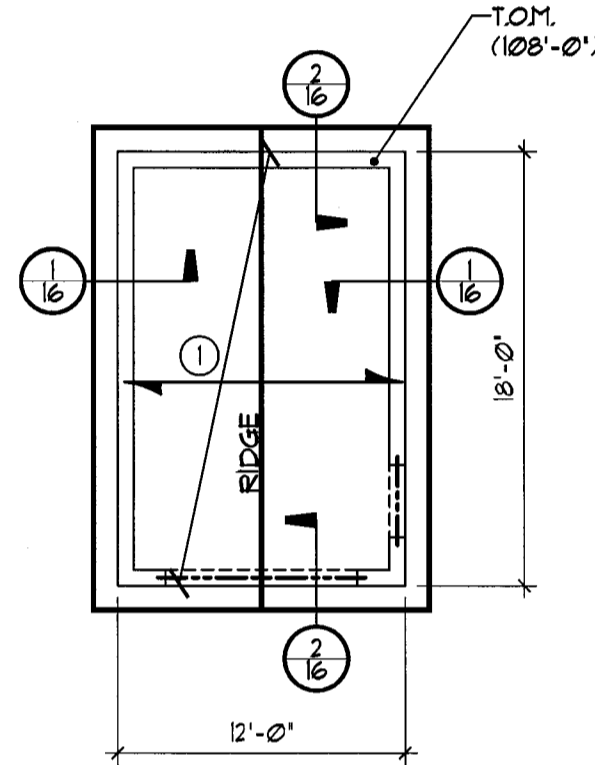
RANGE CONTROL/SAFETY OBSERVATION TOWER FLOOR/ROOF FRAMING PLAN NORTH

SCALE: 1/8" = 1'-0"



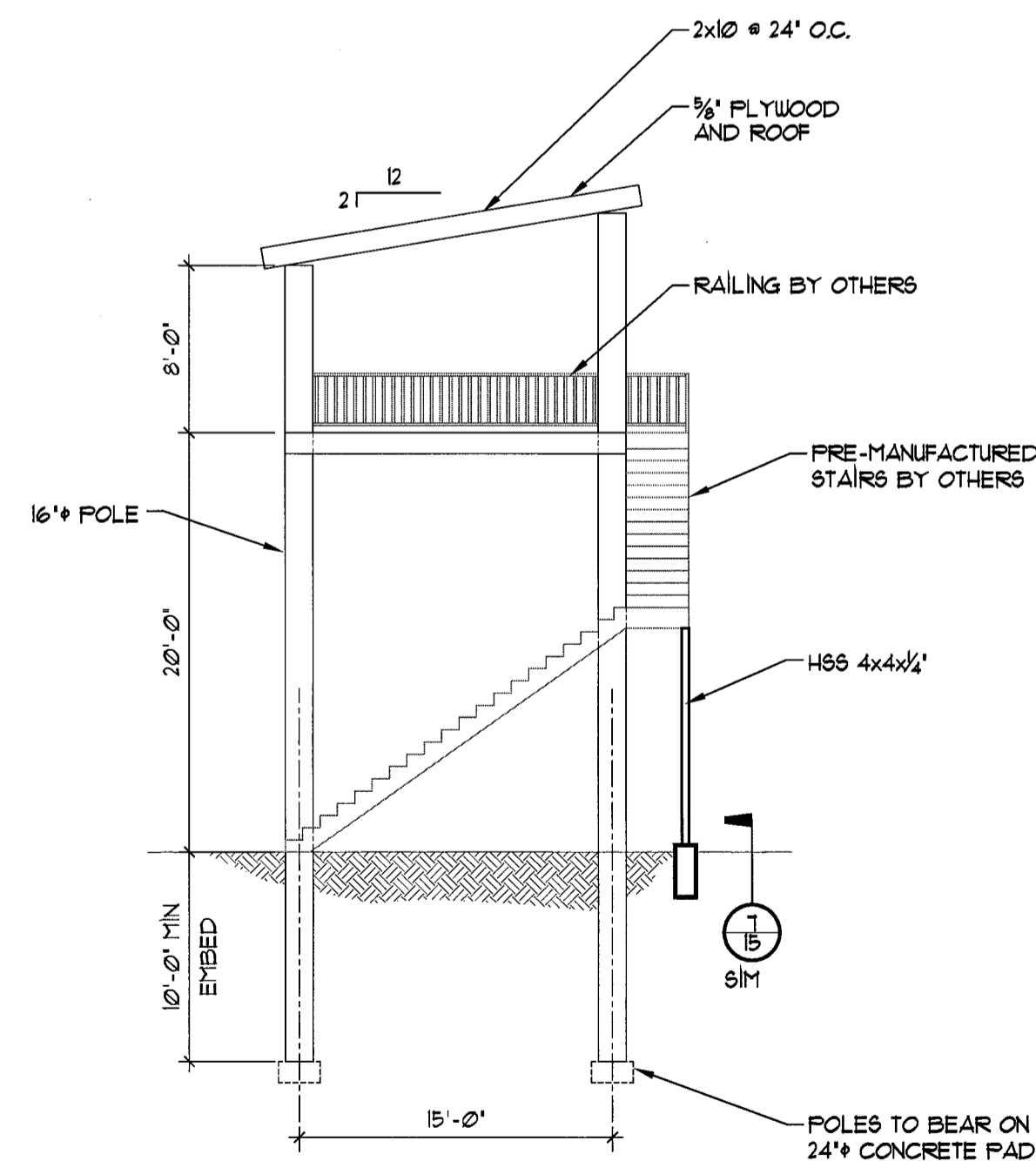
STORAGE SHED FOUNDATION PLAN

SCALE: 1/8" = 1'-0"



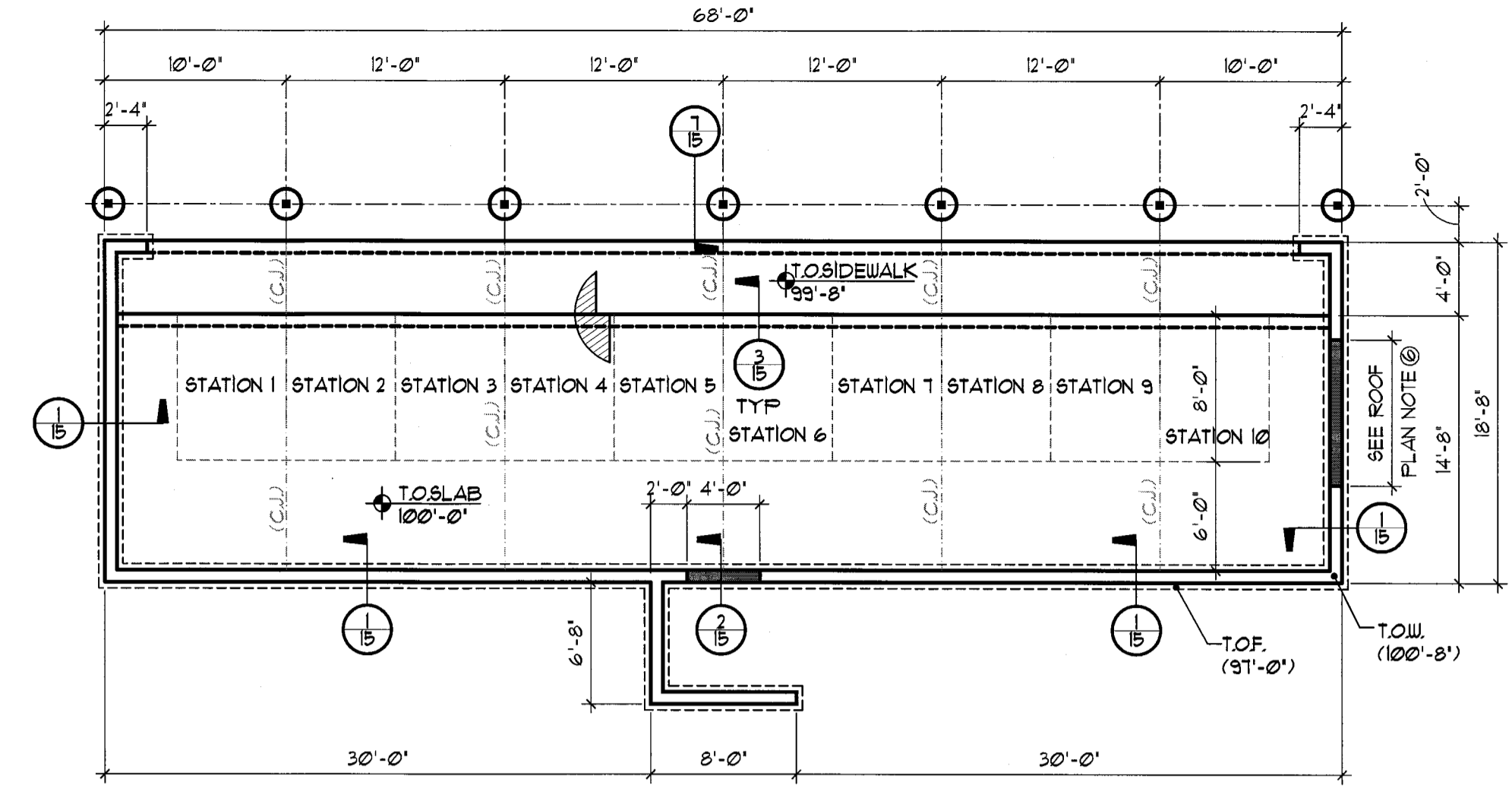
STORAGE SHED ROOF FRAMING PLAN

SCALE: 1/8" = 1'-0"



RANGE CONTROL/SAFETY OBSERVATION TOWER SECTION

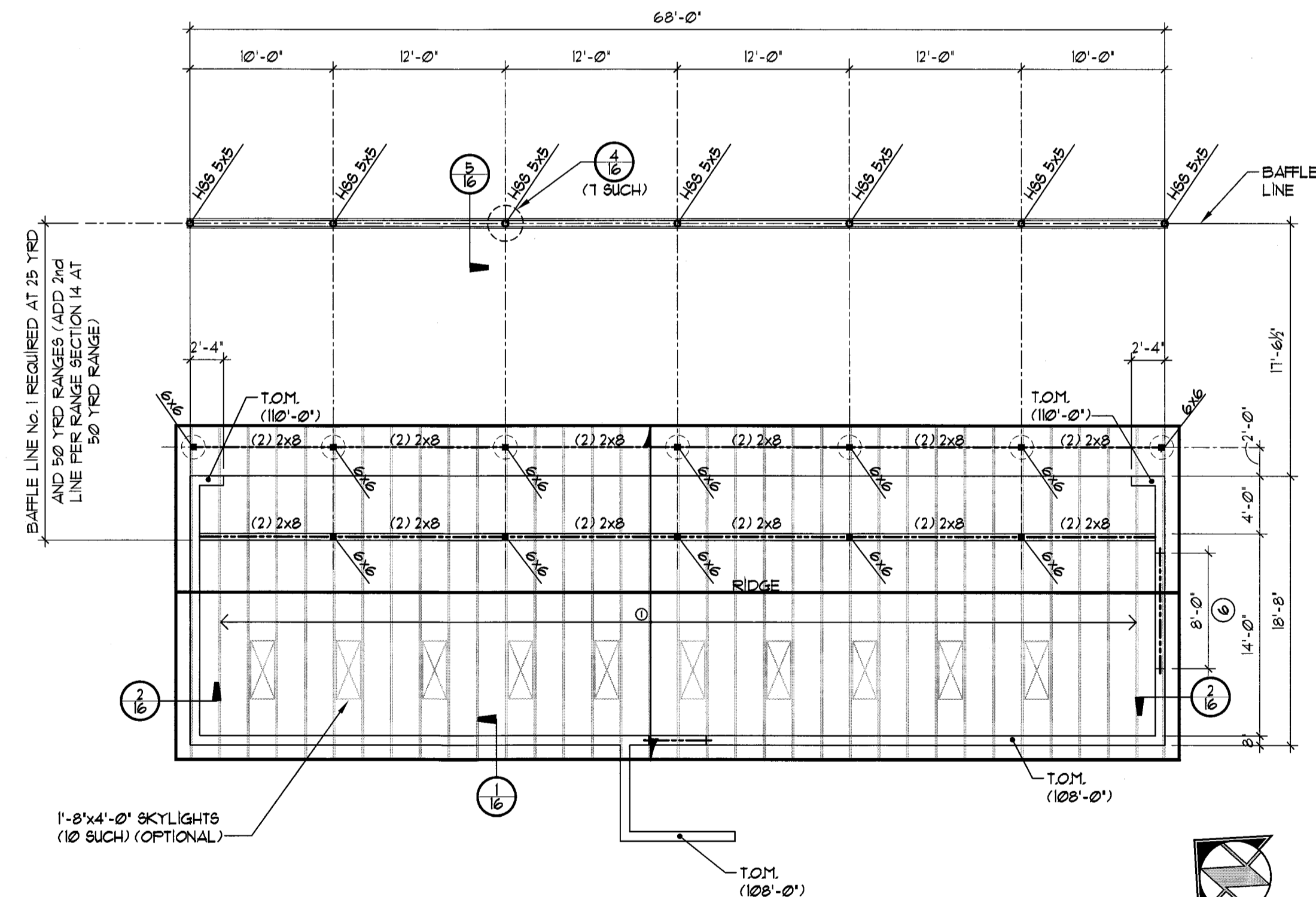
SCALE: 1/8" = 1'-0"



10 STATION FIRING LINE STRUCTURE FOUNDATION PLAN NORTH

SCALE: 1/8" = 1'-0"

FOUNDATION PLAN NOTES	
1. FLOOR IS 4" CONCRETE SLAB ON GRADE REINFORCED WITH CELLULOSE FIBER MESH REINFORCING	5. ALL FOOTINGS ARE CENTERED BELOW COLUMNS OR WALLS UNO
2. ELEVATION TOP OF SLAB = 100'-0", EXCEPT AS NOTED THIS: T.O.SLAB REFER TO CIVIL DRAWINGS FOR ACTUAL ELEVATION	6. ALL CONTINUOUS FOOTINGS ARE 1'-0" DEEP AND 1'-6" WIDE UNO
3. ELEVATION TOP OF CONCRETE FOUNDATION WALLS = 100'-0" EXCEPT AS NOTED THIS: T.O.W (XX'-X"). T.O.W ELEVATIONS SHALL MAINTAIN A MINIMUM OF 6" ABOVE FINISH GRADE	7. FOR TYPICAL SLAB CONTROL JOINT DETAIL RE: 4/14 ALL CONTROL JOINTS (C.J.) TO BE VERIFIED BY CONTRACTOR
4. ELEVATION TOP OF EXTERIOR FOOTINGS = 91'-0" EXCEPT AS NOTED THIS: T.O.F. (XX'-X").	8. FOR CORNER REINFORCING IN 6" WALLS RE: 5/14
	9. SHADE INDICATES DROP WALL 8" AT DOORS AND FOUR THICKENED SLAB THROUGH OPENING



10 STATION FIRING LINE STRUCTURE ROOF FRAMING PLAN NORTH

SCALE: 1/8" = 1'-0"

ROOF FRAMING PLAN NOTES	
1. PRE-ENGINEERED TRUSSES @ 24" O.C.	4. ELEVATION TO OF CHU WALLS NOTED THIS: T.O.M. (X'-XX")
2. ROOF DECK IS 3/4" CDX PLYWOOD W/ 8d NAILS @ 6" O.C. ALONG SUPPORTED EDGES AND 12" O.C. IN FIELD	5. FOR LOOSE LINTEL SCHEDULE RE: 3/16
3. EXTERIOR WALLS ARE LIGHTWEIGHT CMU BEARING WALLS. REFER TO FOUNDATION DETAILS FOR REINFORCING FOR MASONRY CONTROL JOINTS RE: 6/16	6. SIDE ENTRY ON ONE SIDE (ONLY) ADJACENT TO FUTURE BUILDING SITE

GENERAL STRUCTURAL NOTES

1. APPLICABLE CODES

A. These general notes apply to all structural drawings. This project is designed in accordance with the International Building Code (IBC) 2003 Edition and the Minimum Design Loads for Buildings and Other Structures (ASCE 7-02) and The Teller County Building Code (2010 changes).

B. All material and workmanship shall be in accordance with applicable provisions of the codes specified above.

2. LOADS USED IN DESIGN

A. Roof Live Load: P_f (Balanced) 40 psf
(and/or drift provisions ASCE 7-02, section 1.7)
Snow Load Importance Factor, I_s 1.0
Snow Thermal Factor, C_e 1.0
Snow Exposure Factor, C_{ex} 1.0
Roof Dead Load: 5 psf
Floor Live Load: 60 psf
B. Wind: Basic Wind Speed, V 85 mph
Exposure C
Wind Importance Factor, I_w 1.0
Building Category:
I - Other
Internal Pressure Coefficient, GCF 0.85 psf

3. COORDINATION

A. **DO NOT SCALE.** Changes affecting the layout shown must be specific and clearly conveyed to **RMG Engineers Group** in written form as a change for inclusion into these plans. **Contractor and/or client shall verify all dimensions and layout prior to construction.** All dimensions on structural drawings shall be checked against architectural drawings and any discrepancies shall be brought to the attention of the Architect and Engineer immediately. Refer to mechanical, electrical and architectural drawings for openings not shown on structural drawings.

B. Shop drawings shall be prepared by the fabricator. Copying of these construction documents for use as shop drawings will not be permitted. General contractor shall review shops prior to submitting to design team.

C. All temporary shoring shall be the responsibility of the contractor.

D. Design is valid for two years from original date of issue, unless updated to acceptable codes and practices at that time.

E. A preconstruction meeting with personnel of **RMG Engineers Group**, the architect, contractor and appropriate subcontractors is strongly recommended prior to construction to discuss structural plans.

4. CONCRETE

A. Concrete has been designed and shall be constructed in accordance with the American Concrete Institute "Building Code Requirements for Concrete and Specifications for Structural Concrete for Buildings" (ACI 318 and ACI 308) latest editions, Section 13 "Inspection" of ACI 318 is deleted in its entirety, see "Field Observations" paragraph. All concrete shall be of stone aggregate, unless noted otherwise.

B. **Concrete mixes:**
See specifications for any additional durability requirements.

1) 4000 psi for footings, grade beams, and miscellaneous concrete.
4000 psi minimum compressive strength at age of 28 days.

Type III Cement, minimum of 540 pounds per cubic yard.

2) 3400 psi for concrete slabs.
3400 psi minimum compressive strength at age of 28 days.

Type III Cement, minimum of 470 pounds per cubic yard.

3) 3400 psi for concrete slabs.
3400 psi minimum compressive strength at age of 28 days.

4) 3400 psi for concrete slabs.
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5) 3400 psi for concrete slabs.
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6. MASONRY

A. Concrete block walls shall have a minimum ultimate compressive strength (f_m) of 1500 psi. Concrete masonry units (CMU) shall conform to ASTM C90 grade-N made with light weight aggregate. Mortar shall conform to ASTM C270 Type-S. Grout shall conform to ASTM C416 with a minimum compressive strength of 2000 psi in 28 days.

B. Vertical reinforcement shall conform to ASTM A618 grade 60, where splices are required, provide minimum 40% bar diameter lap. Horizontal joint reinforcing shall be standard weight "Dur-O-Wall" or equivalent at 6" o.c. maximum. Provide prefabricated corner and tee sections at wall corners and intersections.

C. Provide a minimum 1/2" vertical bar, or as noted on plan, full height at each door jamb, window jamb, mechanical opening, control joint, or other masonry discontinuity. Locate reinforcing so as to be continuous without interfering with lintel bearings.

D. Special inspection and testing shall be required in accordance with section 1704 of the 2003 International Building Code.

E. Provide cleanouts at the bottom of grouted cells and use high-lift grouting procedures where possible. Mechanical vibrations shall be used to consolidate grout and reconsolidate grout 15 minutes after the initial consolidation. Any masonry in contact with earth shall be grouted full and back parged as required.

7. SPREAD FOOTING FOUNDATIONS

A. The foundation design has been completed in accordance with pertinent standards, recommended design soil parameters, accepted engineering design procedures, and is based on the best information available at the time of completion. The design is intended to minimize differential movement resulting from the heaving of expansive soil or settling of subsurface soils. It must be recognized that foundation components will undergo movement. It shall be the responsibility of the contractor and/or present owner to inform any subsequent owners of the soil condition and advised to maintain good practices in the future with regard to surface and subsurface drainage, framing of partitions above floor slabs, and finish work above the floor slabs, etc.

B. Foundation design is based on soil report No. 12282 prepared by **RMG Engineers Group** dated 10-19-12. The Contractor shall thoroughly review and understand all pertinent construction aspects of this report before beginning any work.

C. Foundation Design parameters include an allowable bearing pressure of 2500 psf with no minimum dead load requirement and with soil preparation per soil report.

D. A representative of the Geotechnical engineer shall observe the open excavation to determine that the soil type and conditions are consistent with design criteria of the soil report. If the soil properties are found to be different from this criteria, the foundation engineer shall be promptly notified so that the foundation design may be reviewed.

E. The contractor shall be responsible to coordinate the location of mechanical openings, floor drains, inserts, depressions, buried cables and utilities, etc. with architectural, civil, mechanical and electrical drawings.

F. Mechanically compact all interior and exterior backfill per geotechnical engineers recommendations. It will also be necessary to adjust and maintain the grade immediately against foundations periodically to avoid the creation of a water trap as the backfill settles over time.

G. Slope backfill away from the building a minimum of 10% for the first 10 feet (2% at paved areas) unless a more stringent requirement is specified by the geotechnical engineer. Carry roof drains across the backfilled areas. Do not allow water to stand or pond near the building. Do not flood the backfill.

H. Contact Geotechnical engineer for proper preparation of subgrade for placement of floor slabs.

I. Floor slabs have a high probability of moving vertically. Floor slabs shall be separated from all structural portions of building with an expansion joint of minimum 1/2" thick Duroflex or other approved joint material. A gap in non-bearing partitions, the non-rigid connections with the slabs and construction and non-rigid construction of door jacks may be required by the geotechnical engineer. If required, these items may also require reconstruction over the life of the structure to maintain the independent movement of the floor slabs.

J. Unless a specific top of wall connection is shown, foundation wall stability is not dependent on floor framing for lateral support. A stud wall (cripple wall) or maximum of (3) 2x4 or (4) 2x6 plates (in addition to embedded mullion) are allowed along the top of the foundation wall (including garden level walls) unless noted otherwise. Walls having backfill on both the interior and exterior faces should have backfill on either side brought up approximately together. Otherwise, where possible, no exterior backfill should be placed until the floor slab is in place or the bottom of the foundation wall is otherwise properly braced. Top of wall must also be braced if backfill is placed within 14 days of concrete pour.

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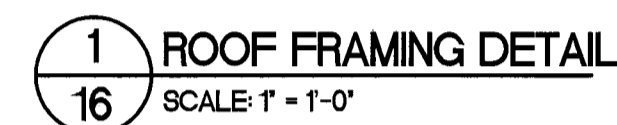
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AP. Unless a specific top of wall connection is shown, foundation wall stability is not dependent on floor framing for lateral support. A stud wall (cripple wall) or maximum of (3) 2x4 or (4) 2x6 plates (in addition to embedded mullion) are allowed along the top of the foundation wall (including garden level walls) unless noted otherwise. Walls having backfill on both the interior and exterior faces should have backfill on either side brought up approximately together. Otherwise, where possible, no exterior backfill should be placed until the floor slab is in place or the bottom of the foundation wall is otherwise properly braced. Top of wall must also be braced if backfill is placed within 14 days of concrete pour.



NOTES:

1. (1) ANGLE FOR EACH 4' WYTHE OF MASONRY
2. REFER TO ARCHITECTURAL & MECHANICAL DRAWINGS FOR LOCATION & SIZE OF OPENINGS
3. BEARING DIMENSIONS SHOWN ABOVE ARE REQUIRED MINIMUMS AT EACH END OF LINTEL
4. USE ABOVE SCHEDULE UNLESS NOTED OTHERWISE ON PLANS
5. GENERAL CONTRACTOR TO COORDINATE HEAD HEIGHTS AND INSTALLATION OF LOOSE LINTELS WITH ARCHITECTURAL DEMO PLANS AND EXISTING CONDITIONS. STRUCTURAL FRAMING STRUCTURE ABOVE IS TO BEAR AT LEAST 1' LINTEL SPAN (OR MORE) ABOVE THE NEW LINTEL IF FIELD LENGTH IS TO BE LESS OR IF MIN LINTEL SPAN CANNOT BE ACHIEVED. CONTACT RYM ENGINEERS GROUP TO DETERMINE IF ADDITIONAL STRUCTURAL SUPPORT IS REQUIRED

