

SKREET & TRAP FIELD LAYOUT PLAN

William Guman & Associates, Ltd.
 URBAN PLANNING | COMMUNITY DESIGN | LANDSCAPE ARCHITECTURE
 731 North Weber Street, Suite 10
 Colorado Springs, CO 80903
 719.633.9700 fax 719.633.4250
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ONLY DRAWINGS WHICH BEAR THE OFFICIAL STAMP OF THE LICENSED LANDSCAPE ARCHITECT IN THE STATE OF COLORADO ARE VALID FOR CONSTRUCTION PURPOSES.

Gold Camp Shooting Sports Center
County Road 81
 TELLER COUNTY, CO

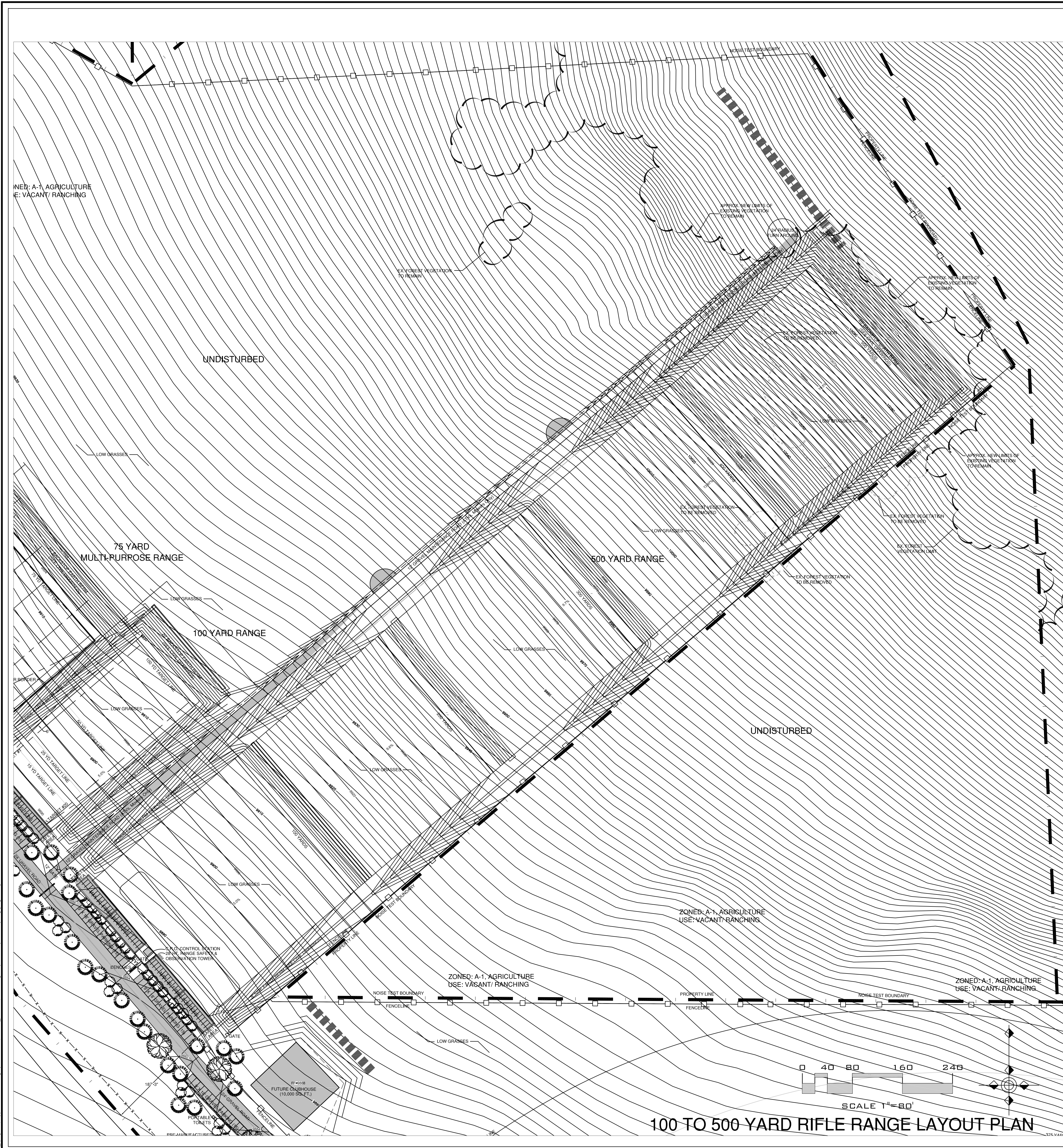
DATE: 11/5/2012
 DRAWN: WFB
 CHECKED: JRA

REVISIONS:		
DATE:	BY:	COMMENTS:
06.17.2013	wfg	Staff Report comments added

REVISIONS:		
DATE:	BY:	COMMENTS:

SKREET & TRAP FIELD PLAN

SHEET NO.
2
 OF 16 SHEETS



LANDSCAPE & REVEGETATION

LANDSCAPE PLANTING SCHEDULE

Planting Schedule:

CODE	BOTANICAL/COMMON NAME	LOW WATER	MATURE HT./WD.	PLANTING SIZE	NOTES
DECIDUOUS TREES					
BO	BUR OAK (<i>Quercus macrocarpa</i>)	☞	50-80'	2.0' CAL.	B&B Nursery Grown, Size to meet or exceed AAN, min. size
QA	QUAKING ASPEN (<i>Populus tremuloides</i>)	☞	30-40' 20-30'	6-8' HT.	Collected from on site. To meet or exceed AAN, min. size
EVERGREEN TREES					
PO	PONDEROSA PINE (<i>Pinus ponderosa</i>)	☞	20-40' 10-20'	4 - 6.0' HT	Collected from on site. To meet or exceed AAN, min. size

☞ **PLANTS WITH LOW WATER REQUIREMENTS AFTER ESTABLISHMENT**

Landscape Symbol Key:

SYMBOL	DESCRIPTION/REMARKS
PER PLAN	GRASS MIX 40% Tallgrass Annual Forage Ryegrass 20% Ryegrass VNS 15% Kentucky Bluegrass 10% Potomac Orchardgrass 05% Smooth Bromegrass 05% Timothy Grass 05% Alsike Clover SOW RATE: 0.6 to 0.7 pounds per 1,000 square feet, 30 pounds per acre.
PER PLAN	COBBLE: To be 1-2" River Rock placed to a depth of 4" on fabric underlayment.
PER PLAN	TIMBER BORDER: Standard Rail Road Tie Timbers buried to separate cobble and turf areas where indicated on the drawings.

WORK AREA PRESERVATION NOTES:

- All existing landscaping that is to be preserved shall be securely fenced by the Contractor to protect existing site vegetation from damage or destruction. Contractor shall prevent vehicular and other construction traffic from encroachment into the preservation area at all times throughout duration of construction and up until Final Acceptance by Architect.

EXISTING TREE PRESERVATION NOTES:

- Critical root zone (CRZ) of a tree shall be defined as the ground area around the tree trunk and extends to the tree's dripline.
- The tree preservation measures will be done by the Contractor:
 - All trees to remain shall be pruned structurally for ground clearance as deemed necessary.
 - Tree roots shall only be pruned to the depth of any proposed grading or soil disturbance.
 - Storage of any materials, vehicles or equipment, or dumping of any contaminants within a CRZ shall be prohibited.
 - Filling over a CRZ shall be prohibited without an Architect-approved mitigation plan.
 - Prior to any trenching or excavation in a CRZ, tree roots shall be pruned at the edge of the proposed trench or excavation, however, no excavation is permitted within 20' of any existing tree trunk.
 - All work on trees to remain, including crown and root pruning shall be performed by a licensed tree service contractor.
 - All dead wood and damaged limbs shall be pruned out by the completion of construction.
 - Existing Ponderosa Pine and Aspen trees that can safely be moved with a mechanical tree spade or balled & burtopped may be used to satisfy the tree planting requirements as indicated per plan.

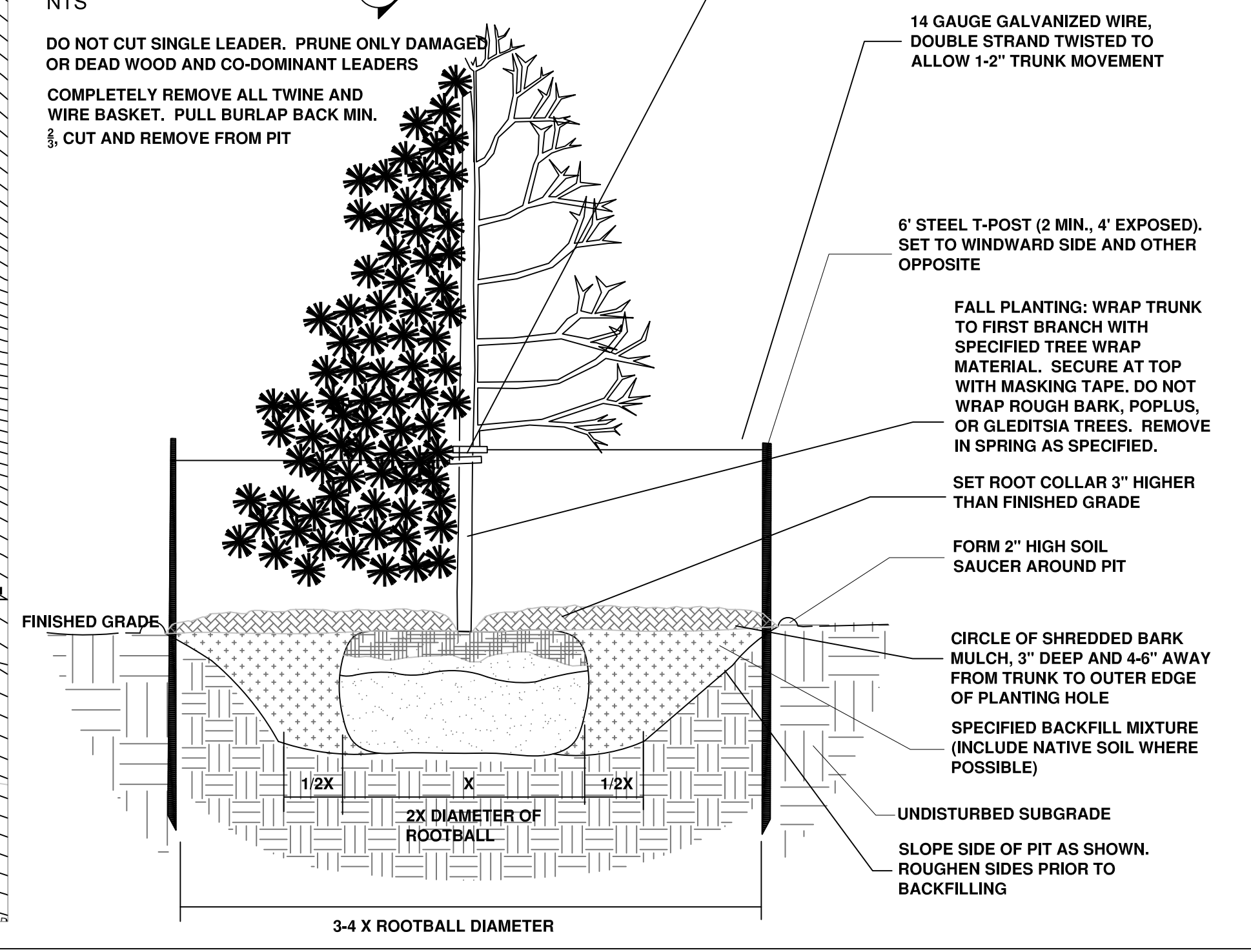
LANDSCAPE NOTES:

- Construction site shall be maintained in a neat, clean, and healthy condition. Maintenance shall include proper pruning, weeding, removal of litter, fertilizing, replacement of dead plants, and manual irrigation of all live landscaping.
- All trees to have 36" mulch ring per details with 3" depth wood mulch.
- Drawings are diagrammatic. Precise placement of components may not be possible as indicated. Consult Project Architect prior to making random field changes which may alter design intent.
- Quantities are provided for reference only. Verify all quantities prior to submitting cost proposal. In the event of a conflict between stipulated, implied, or expressed quantities, those quantities which can be determined graphically from the drawings will prevail in any order.
- The contractor is responsible for inspection and verification of all field conditions and resolving conflicts pertaining to dimensions, layout, etc., which may affect the landscape installation. Mobilizing shall be construed as acceptance of conditions.
- The Landscape Architect reserves the right to consider and approve alternate installations at any time which in the Landscape Architect's opinion maximizes the construction budget and maintains design intent.

LANDSCAPE MAINTENANCE REQUIREMENTS:

- All newly planted landscaping shall be maintained by Teller County Shooting Society and shall include but may not be limited to: a hand watering of all trees & seeded areas to ensure proper establishment.

TREE PLANTING DETAIL



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 REGISTERED ARCHITECT, PROJECTS OF LANDSCAPE ARCHITECTURE

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06.17.2013	wfg	Staff Report comments added

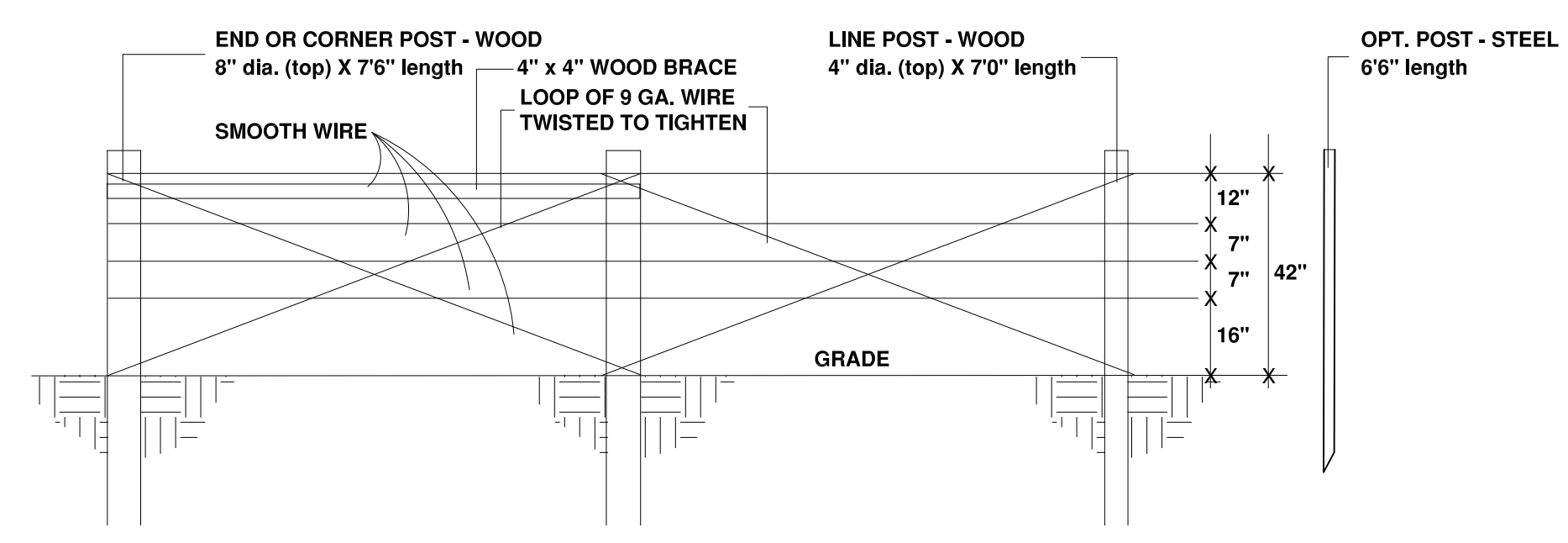
DATE:	BY:	COMMENTS:

100 TO 500 YARD RIFLE RANGE PLAN

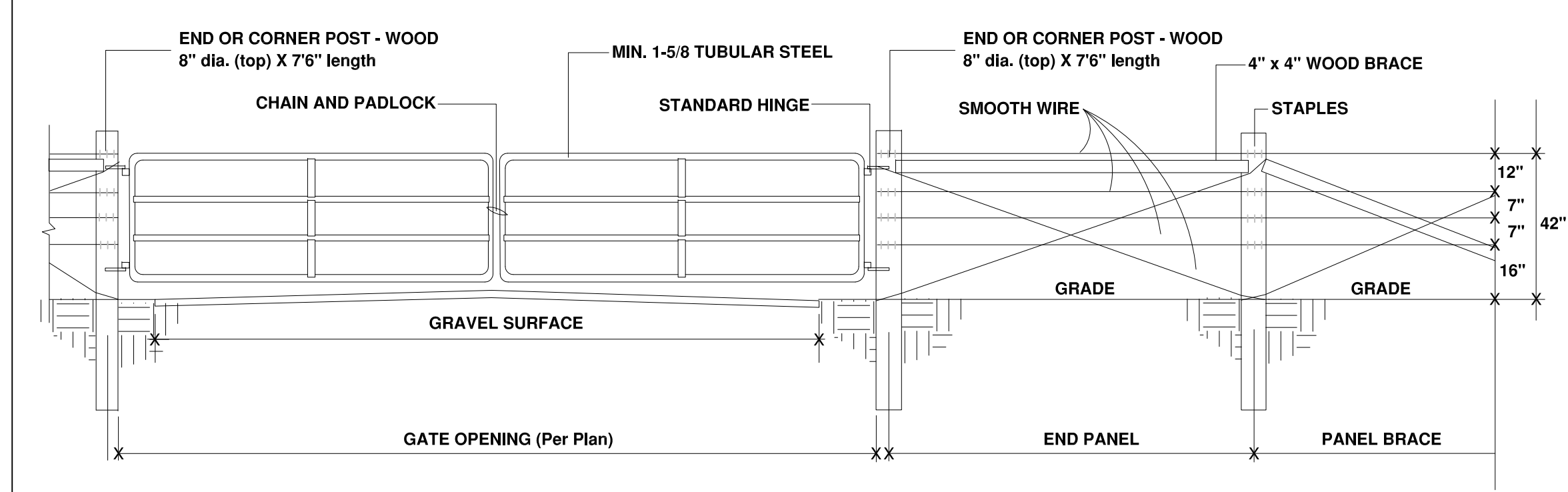
SHEET NO. **3** OF 16 SHEETS



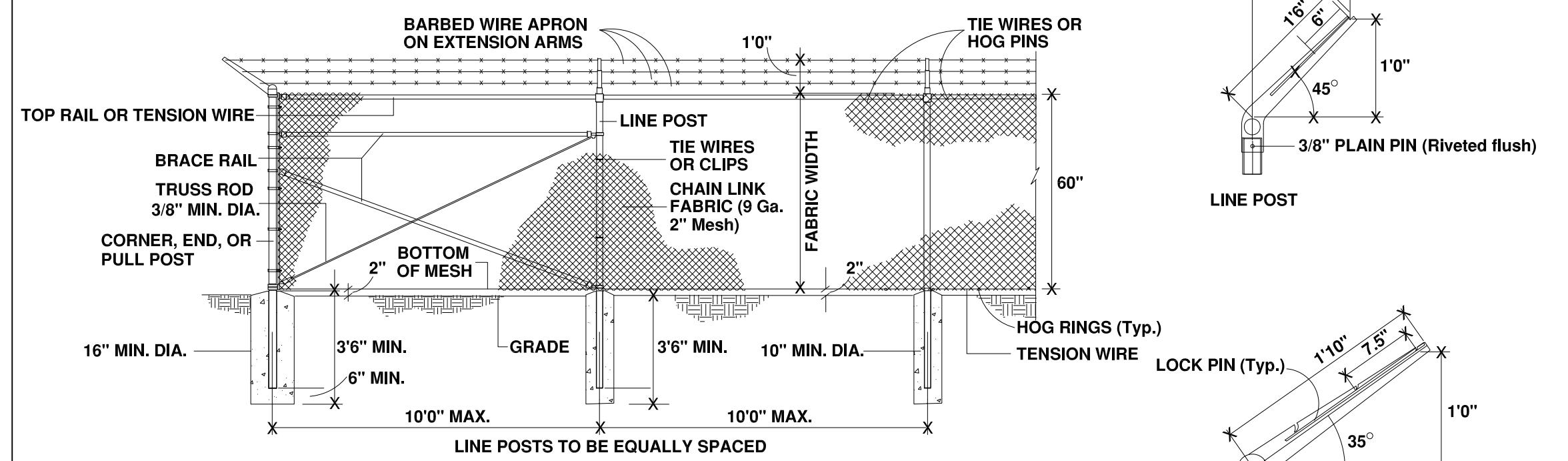
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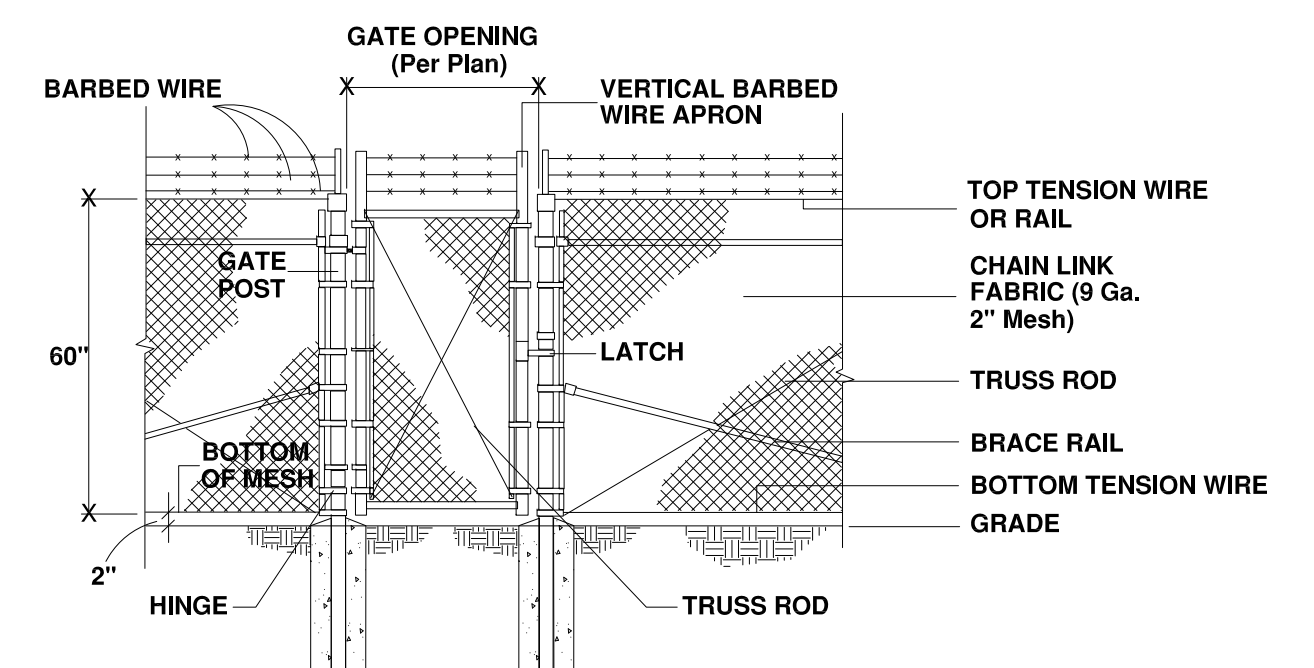
FENCELINE DETAIL (BOUNDARY FENCING) 4-STRAND SMOOTH WIRE NTS



TYPICAL PANEL GATE (BOUNDARY FENCING) NTS



CHAIN LINK SECURITY FENCE (SAFETY AREAS AND STRUCTURE ENTRANCES) NTS



CHAIN LINK SECURITY GATE (SAFETY AREAS AND STRUCTURE ENTRANCES) NTS

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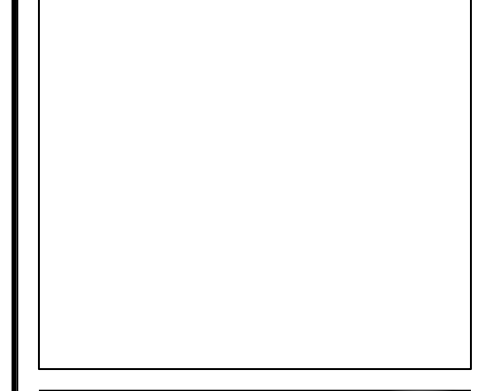
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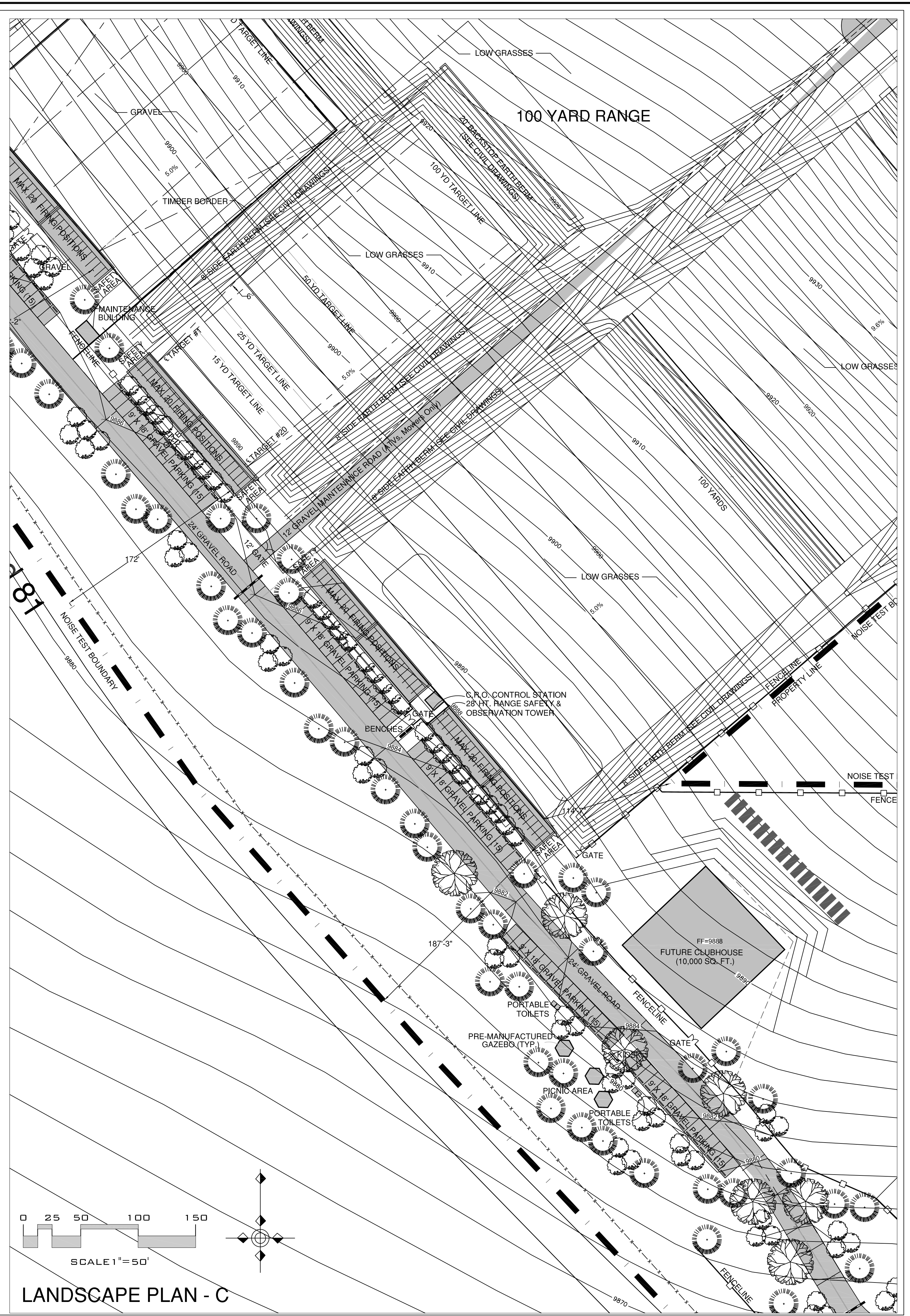
DATE: 11/5/2012
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 CHECKED: JRA



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06.17.2013	wfg	Staff Report comments added

ARCHERY AND 25-100 YD RANGE FENCE DETAILS

SHEET NO. **4** OF 16 SHEETS



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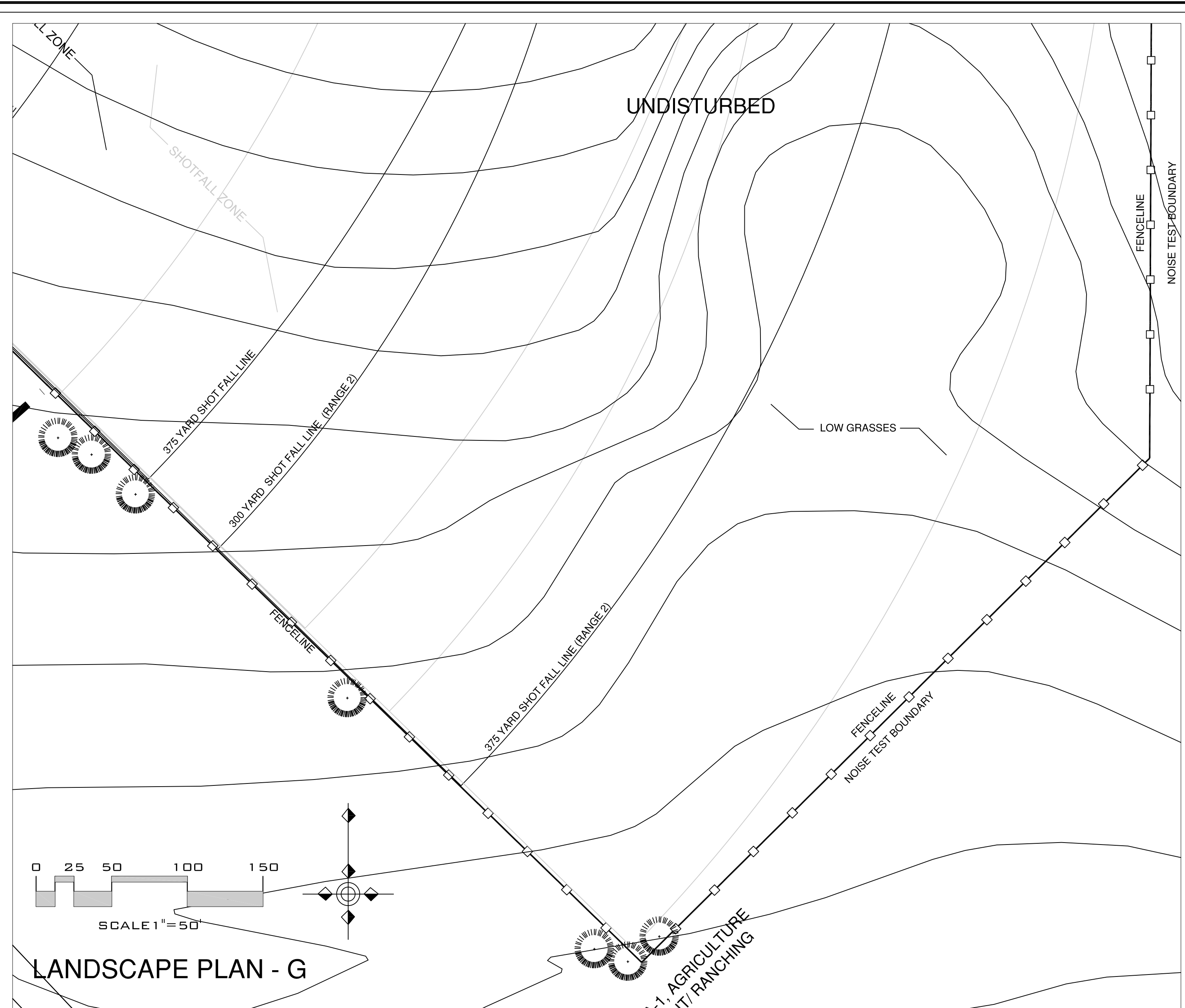
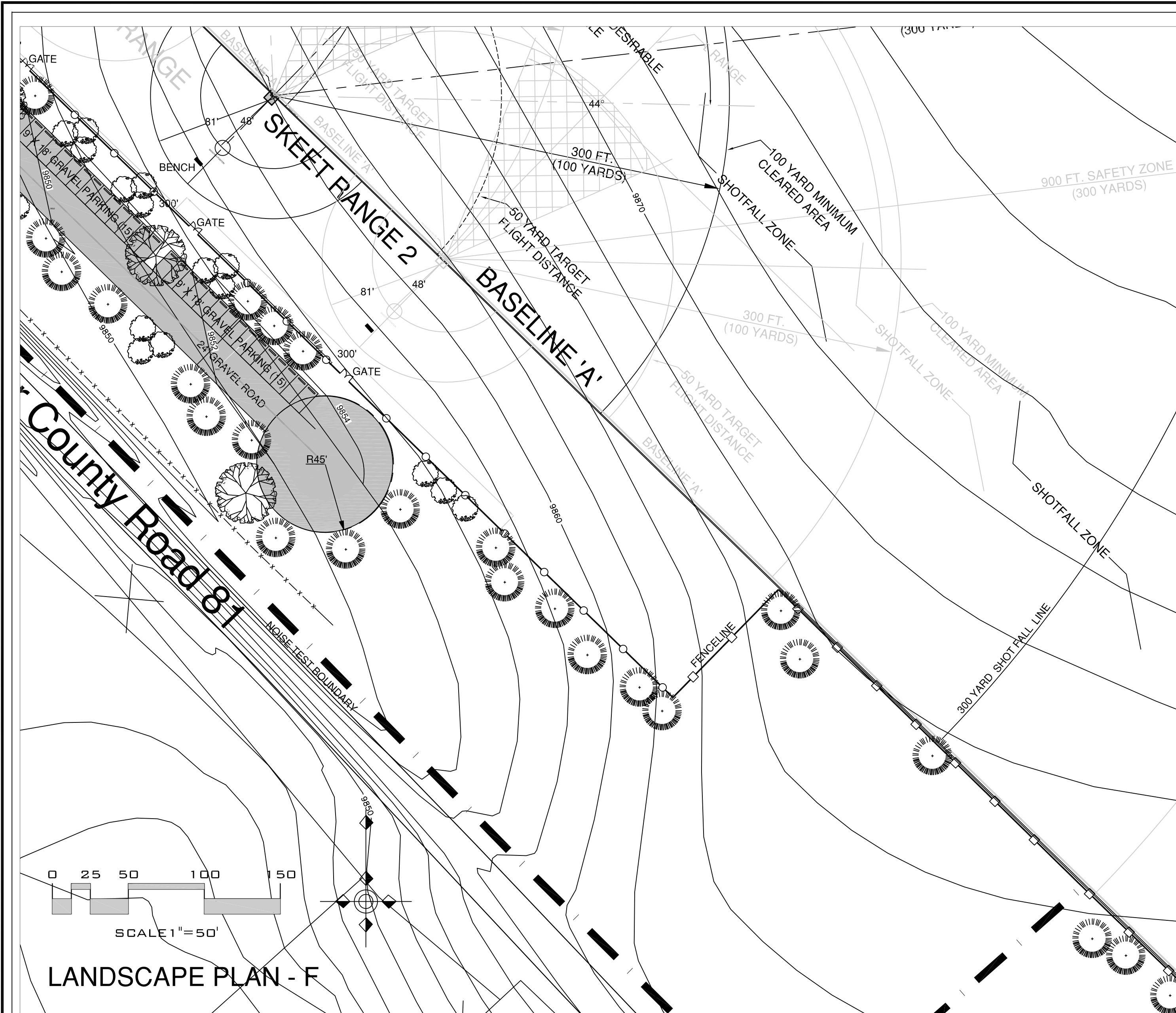
FF-9688
 FUTURE CLUBHOUSE
 (10,000 SQ. FT.)

REVISIONS:		
DATE:	BY:	COMMENTS:
06.17.2013	wfg	Site Report comments added

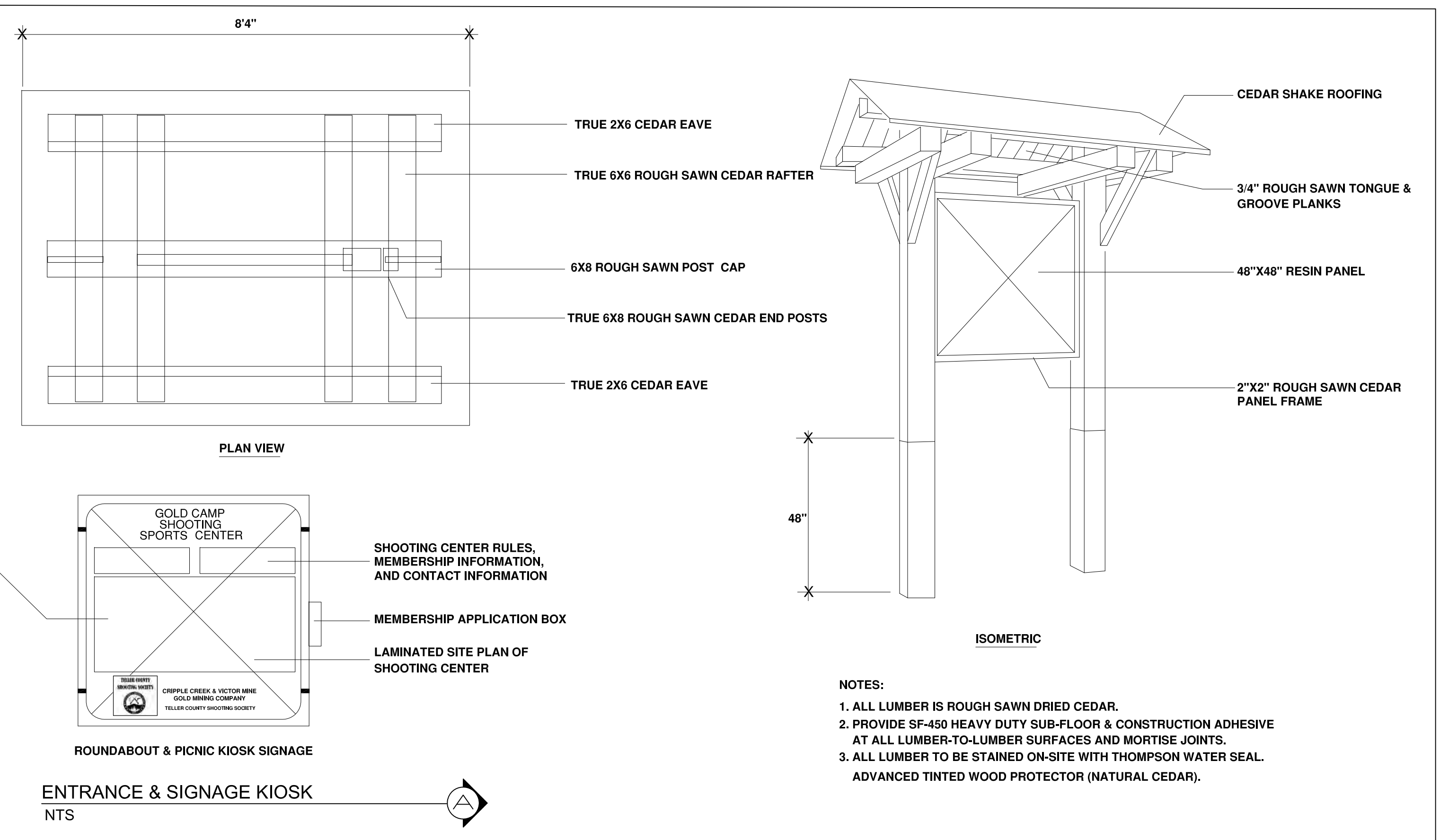
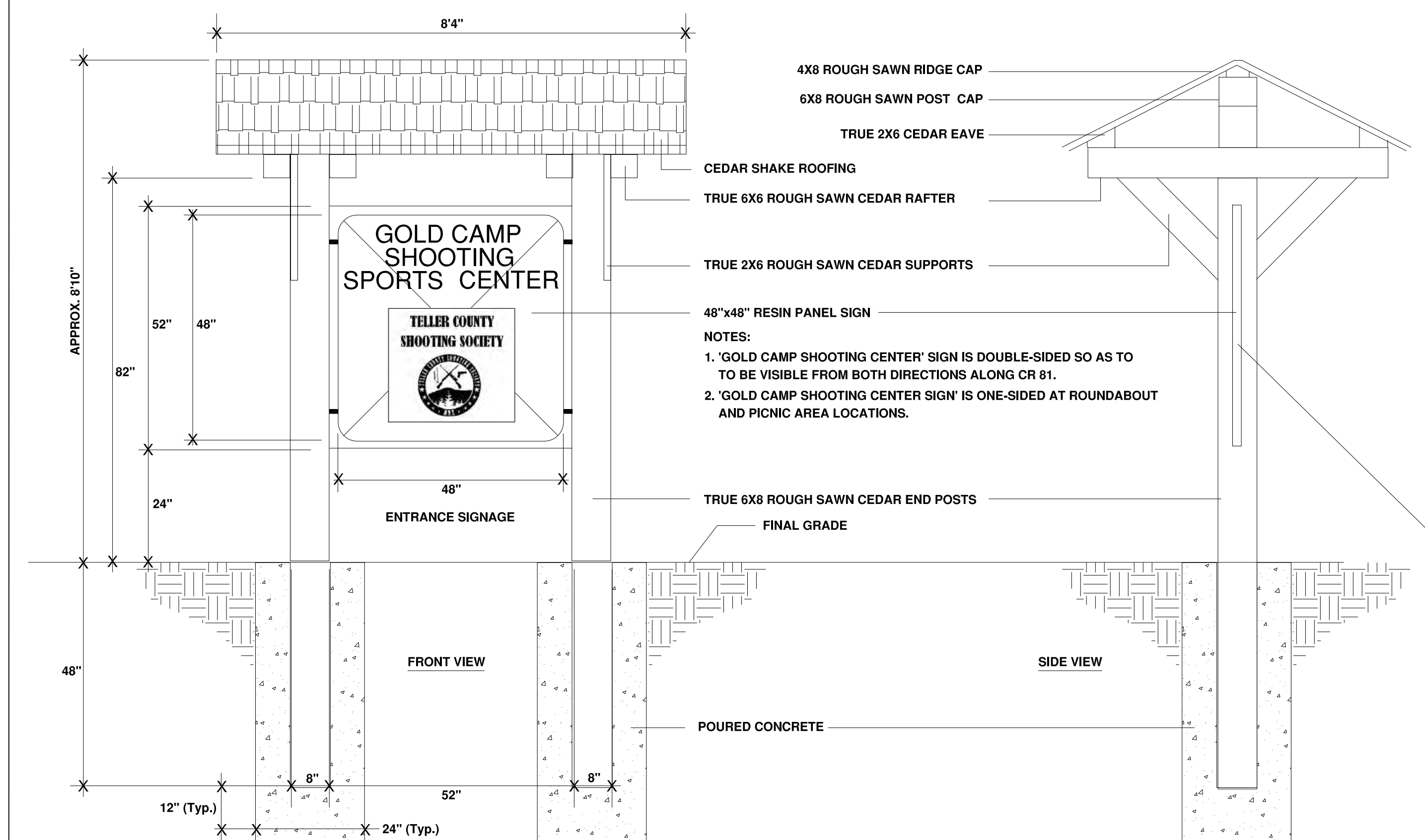
ARCHERY AND
 15 TO 100 YARD
 PISTOL RANGE

SHEET NO.
5
 OF 16 SHEETS

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LANDSCAPE DETAILS & NOTES:



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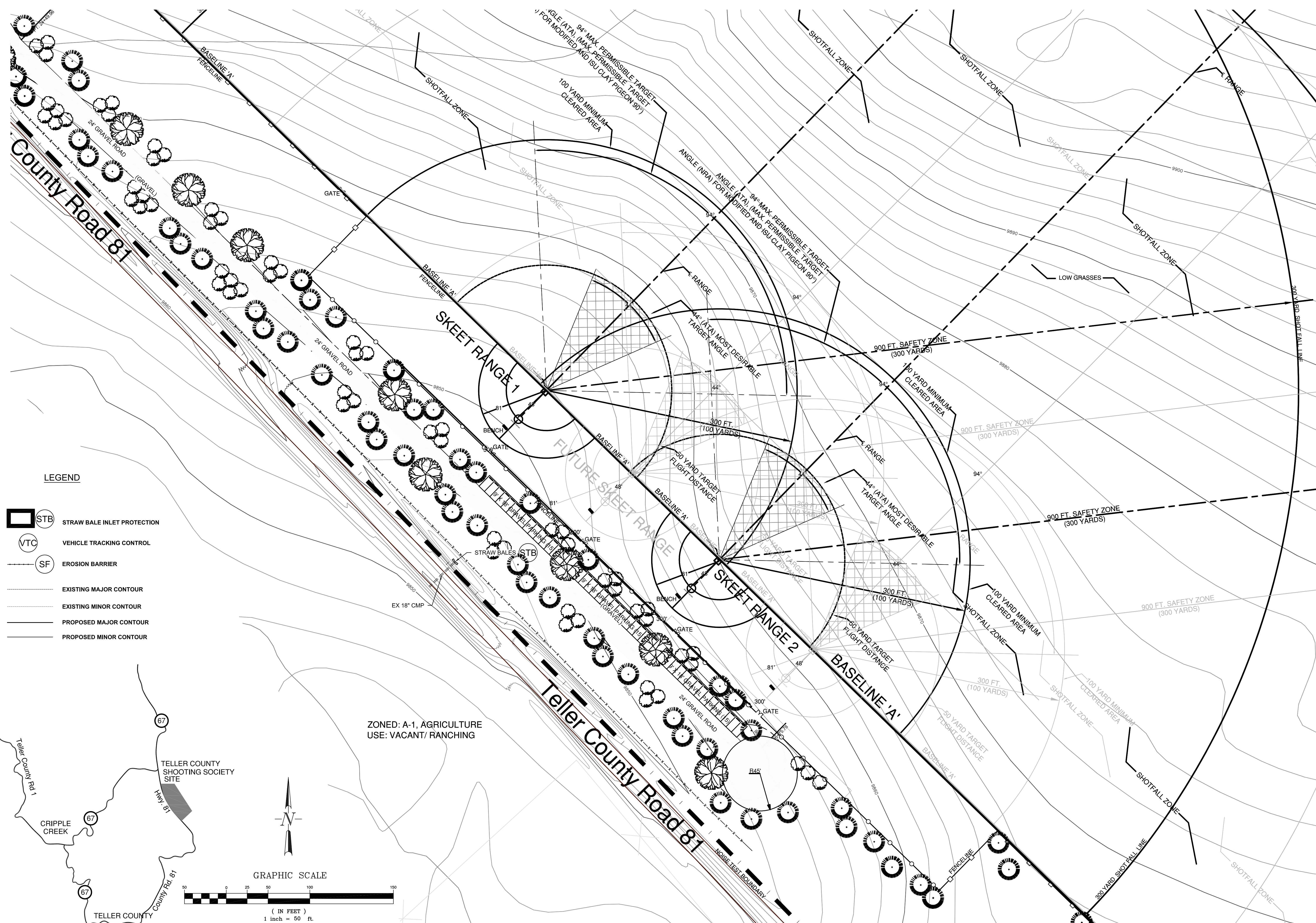
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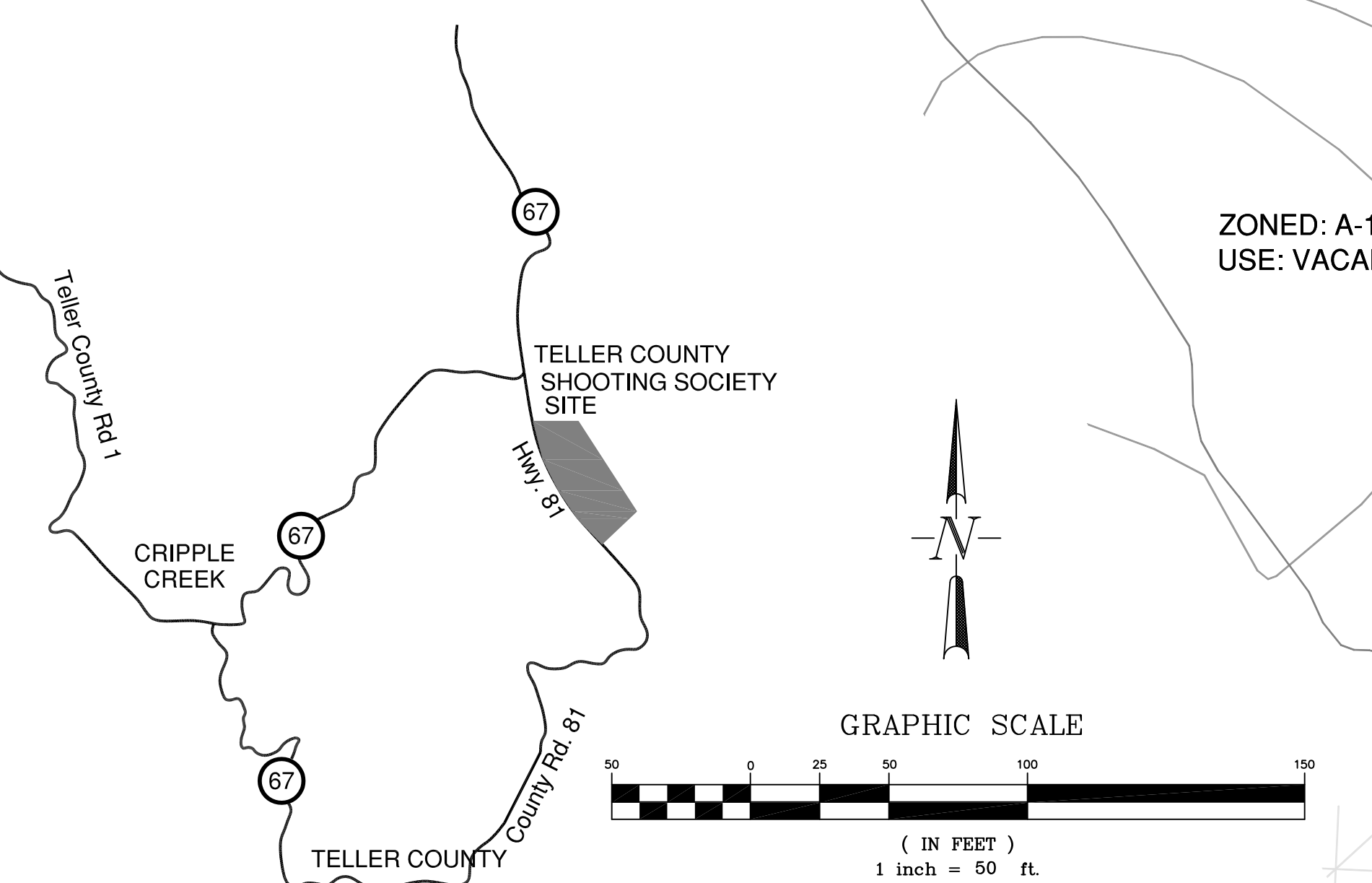
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06.17.2013	wfg	Staff Report comments added

SKEET AND TRAP PLAN KIOSK DETAILS



- LEGEND**
- STRAW BALE INLET PROTECTION
 - VEHICLE TRACKING CONTROL
 - EROSION BARRIER
 - EXISTING MAJOR CONTOUR
 - EXISTING MINOR CONTOUR
 - PROPOSED MAJOR CONTOUR
 - PROPOSED MINOR CONTOUR

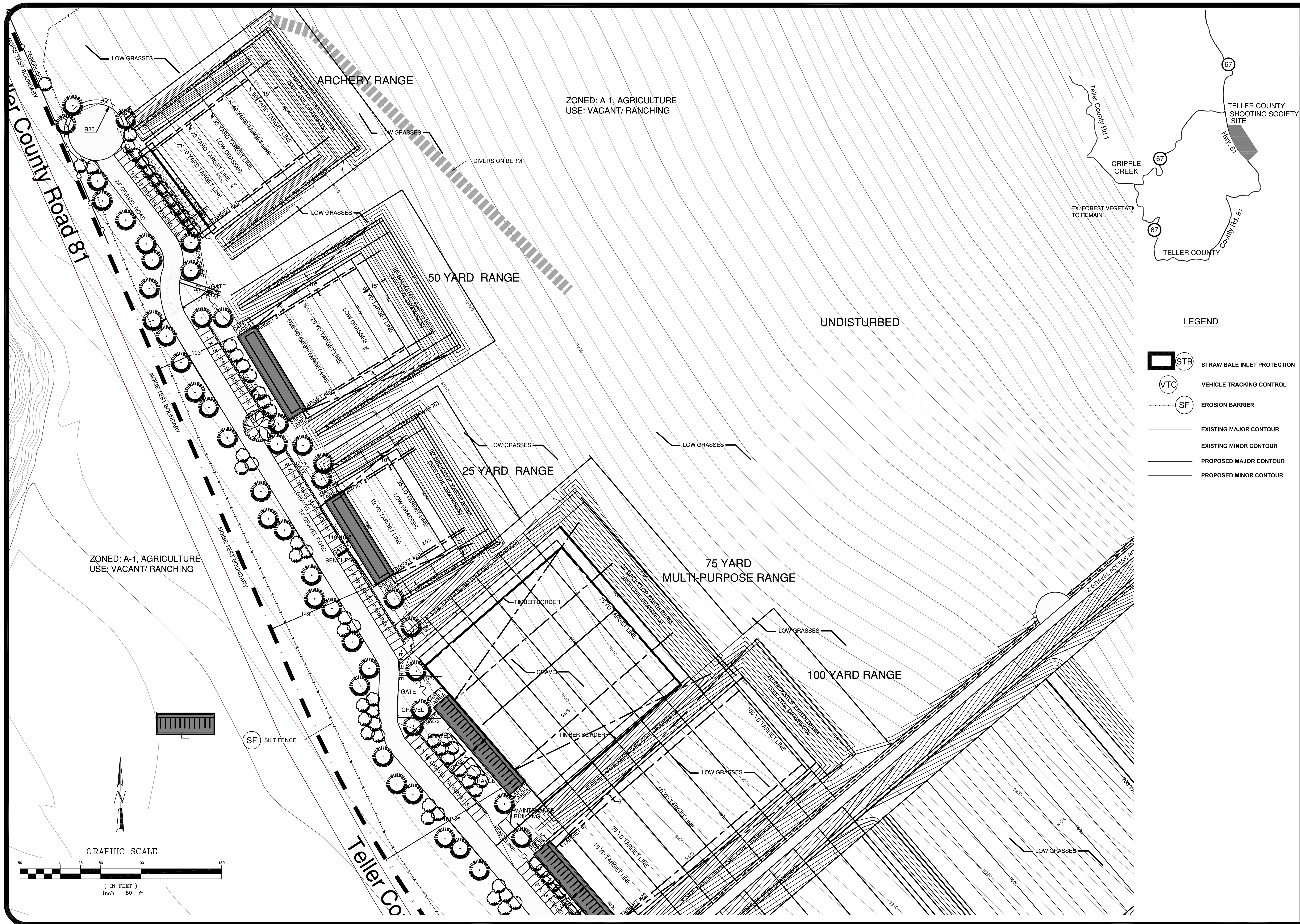


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 PROJECT ENGINEER: MAB
 PROJECT MANAGER: MAB
 CAD FILE NO.: 111008
 DATE: 8/17/13
 SCALE: 1" = 50'
 DRAWN BY: JRH
 VER.:

PREPARED BY:
ADP
 Associated Design Professionals, Inc.
 3520 Austin Bluffs Parkway
 Suite 230
 Colorado Springs, CO 80918
 (719) 266-5212
 fax: (719) 266-5341

NO.	DATE	REVISION	BY

GOLD CAMP SHOOTING SPORTS CENTER
COUNTY ROAD 81
TELLER COUNTY, COLORADO
GRADING/EROSION CONTROL PLAN

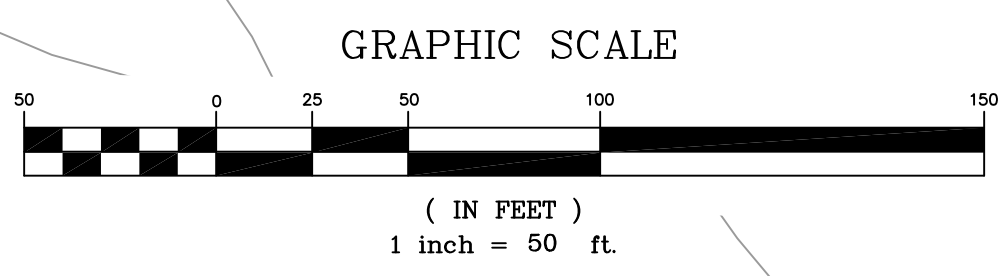
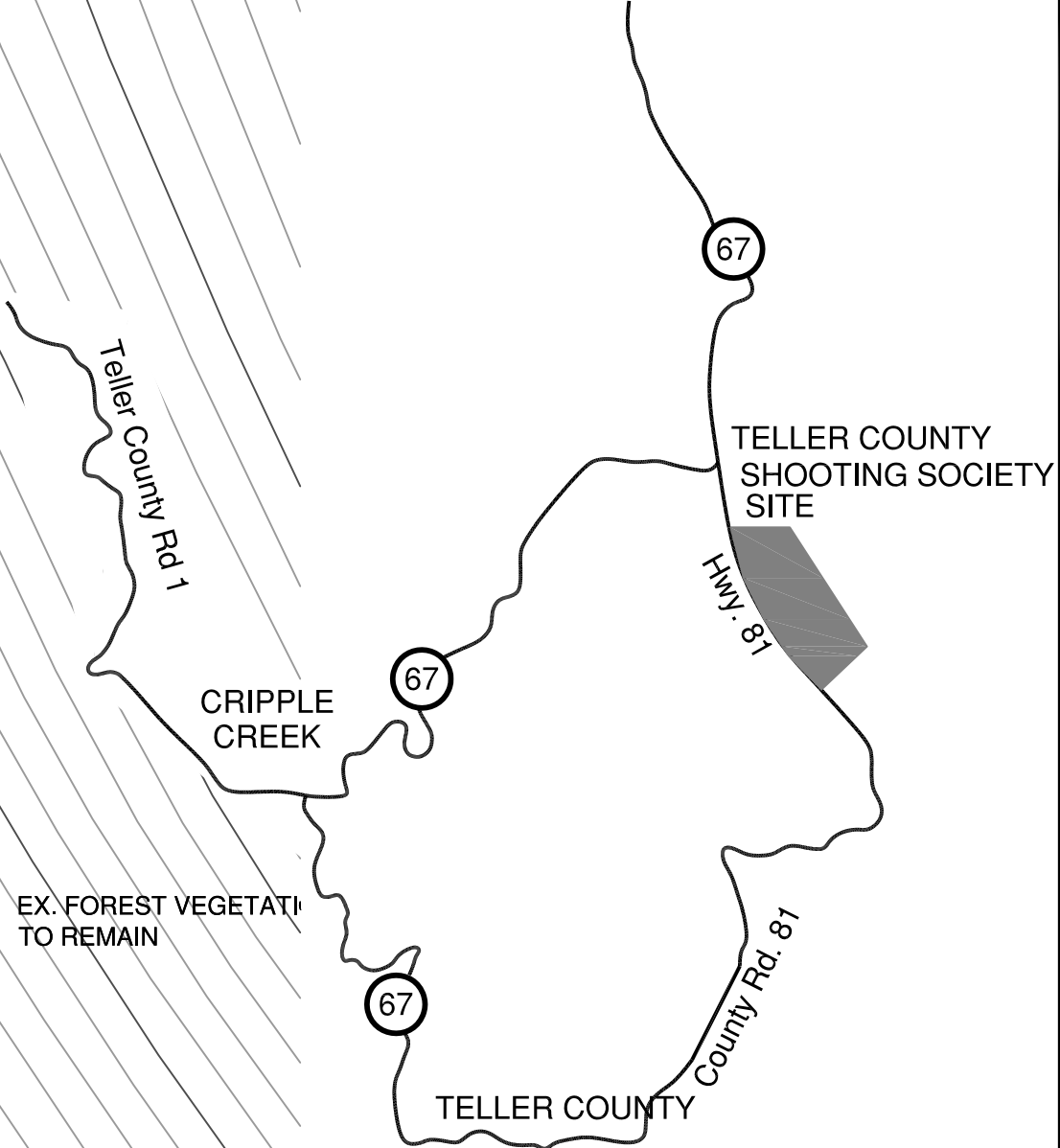


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USE: VACANT/ RANCHING

ZONED: A-1, AGRICULTURE
USE: VACANT/ RANCHING

LEGEND

- STRAW BALE INLET PROTECTION
- VEHICLE TRACKING CONTROL
- EROSION BARRIER
- EXISTING MAJOR CONTOUR
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- PROPOSED MAJOR CONTOUR
- PROPOSED MINOR CONTOUR

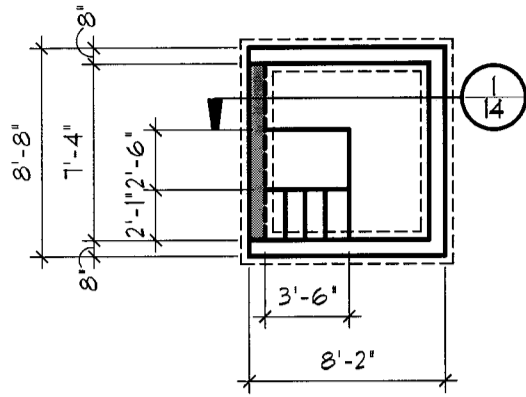


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PROJECT ENGINEER: MAB
PROJECT MANAGER: MAB
DATE: 8/17/13
JOB NO.: 111006
CAD FILE NO.: MAB
DRAWINGS: MAB
SCALE: 1" = 100'
DRAWN BY: JRH
VER: 1

PREPARED BY:
ADP
Associated Design Professionals, Inc.
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Suite 200
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(719) 266-5212
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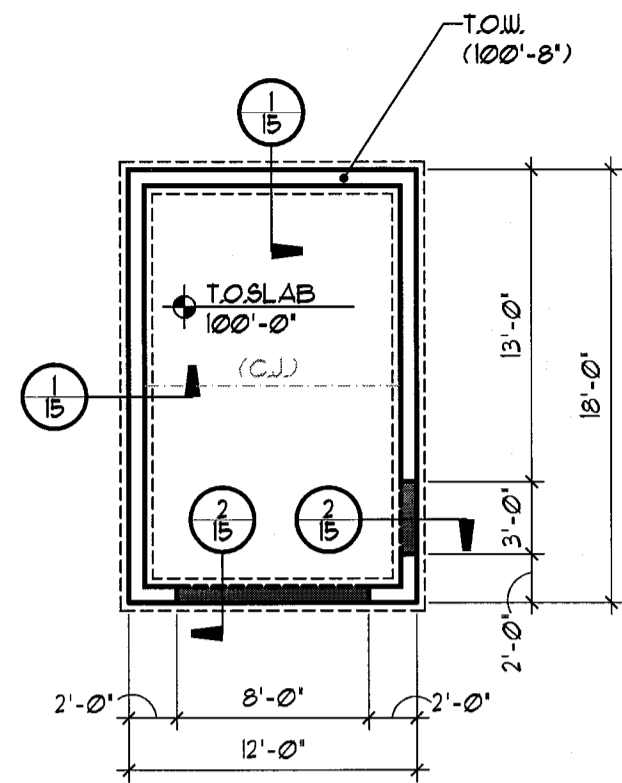
NO.	DATE	REVISION	BY

GOLD CAMP SHOOTING SPORTS CENTER
COUNTY ROAD 81
TELLER COUNTY, COLORADO
GRADING/EROSION CONTROL PLAN



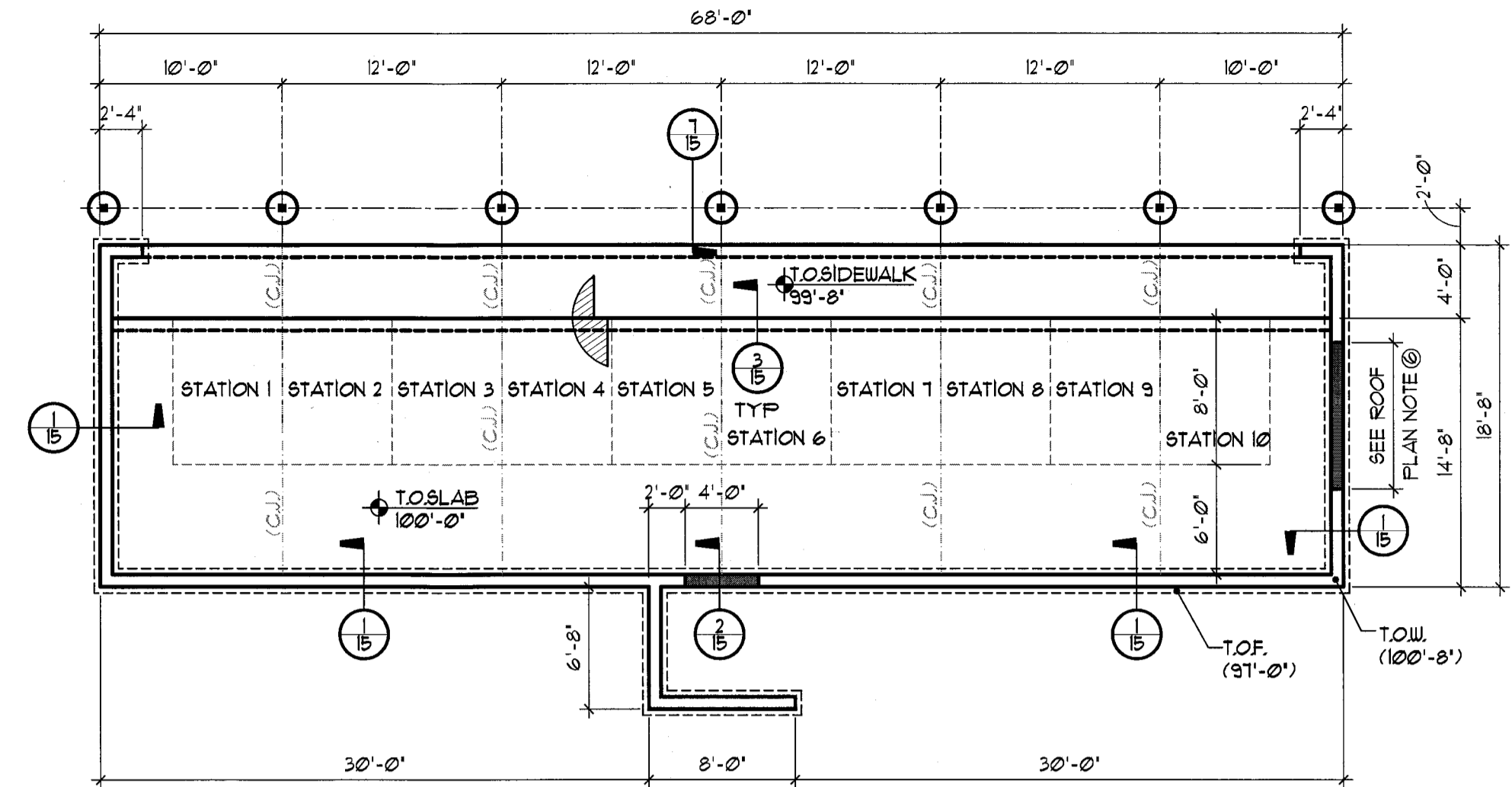
TRAP HOUSE FOUNDATION PLAN NORTH

SCALE: 1/8" = 1'-0"



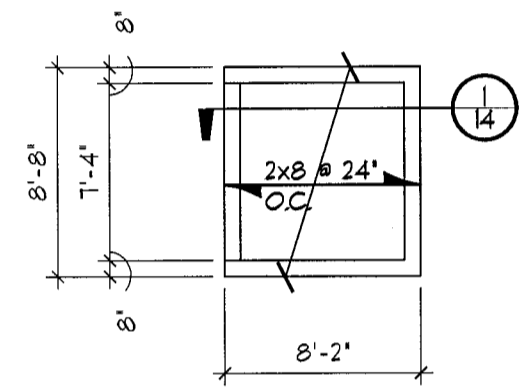
STORAGE SHED FOUNDATION PLAN

SCALE: 1/8" = 1'-0"



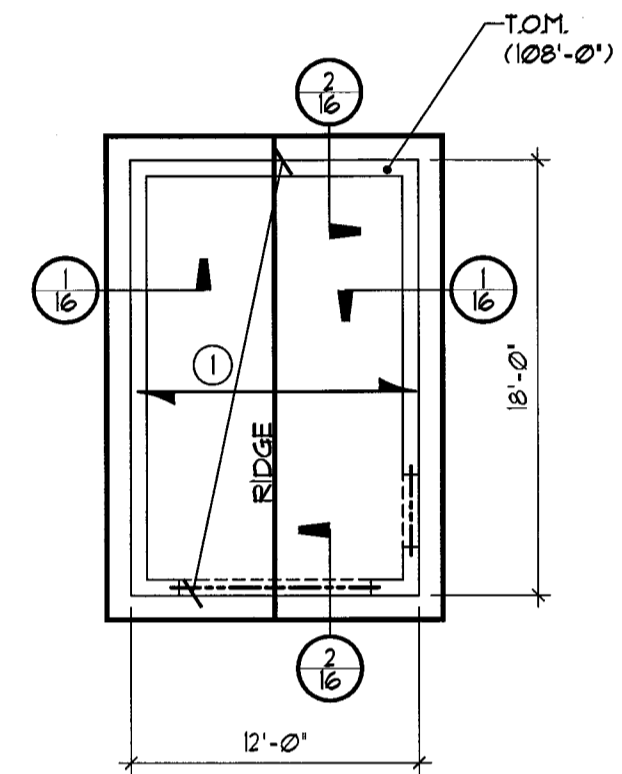
10 STATION FIRING LINE STRUCTURE FOUNDATION PLAN NORTH

SCALE: 1/8" = 1'-0"



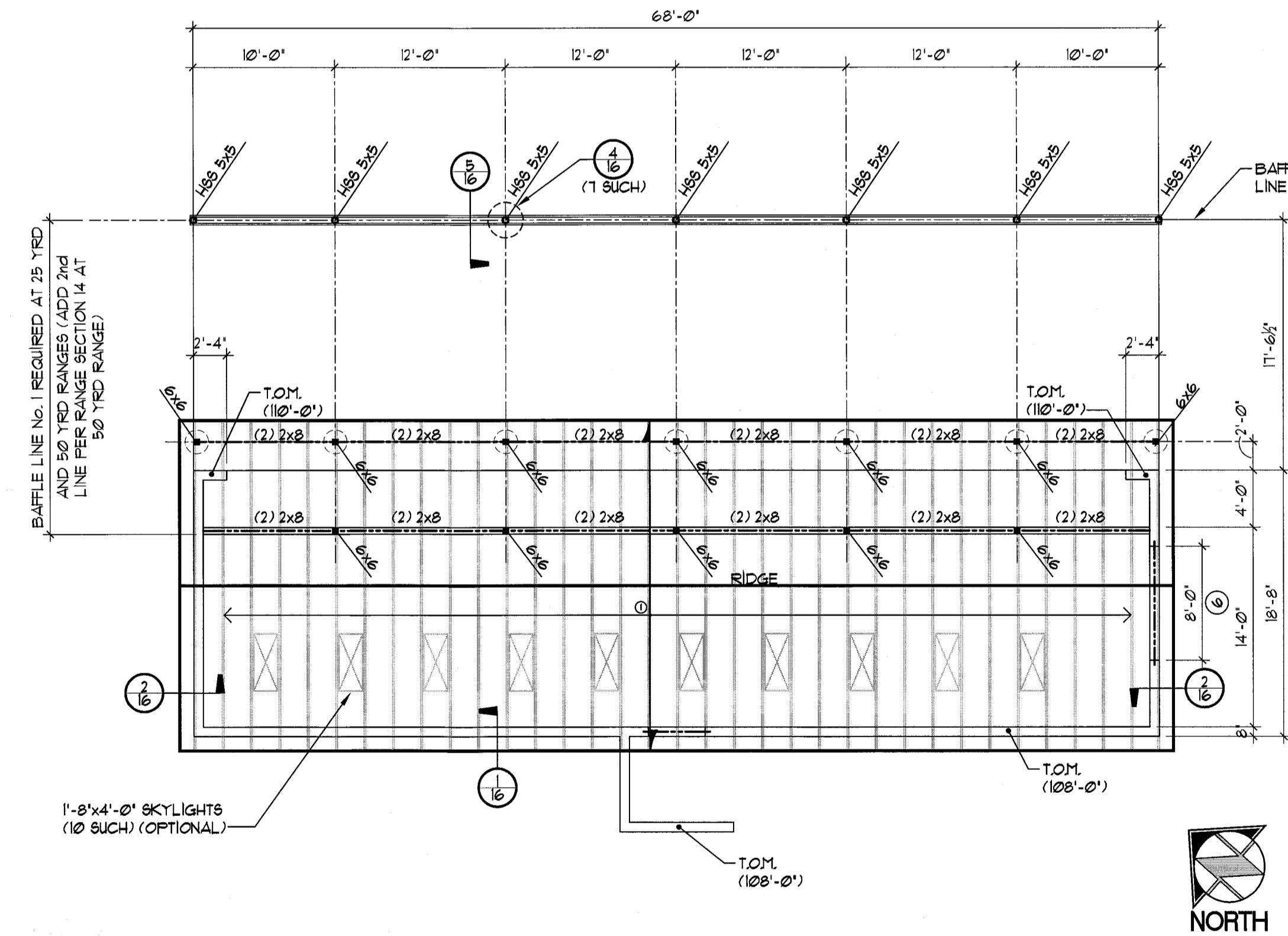
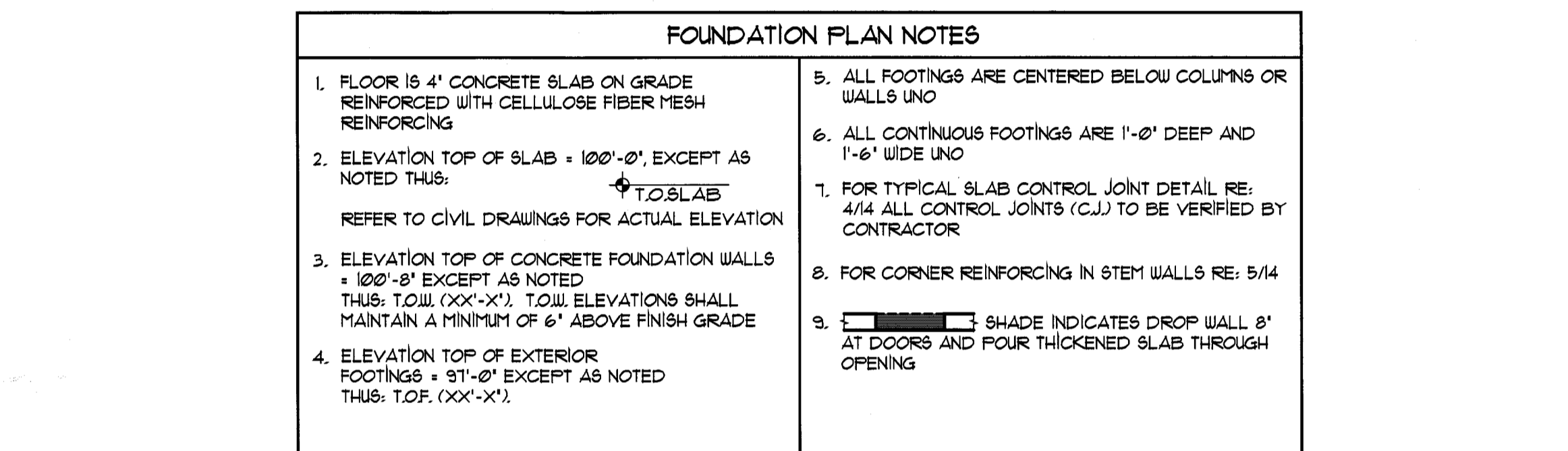
TRAP HOUSE ROOF FRAMING PLAN

SCALE: 1/8" = 1'-0"



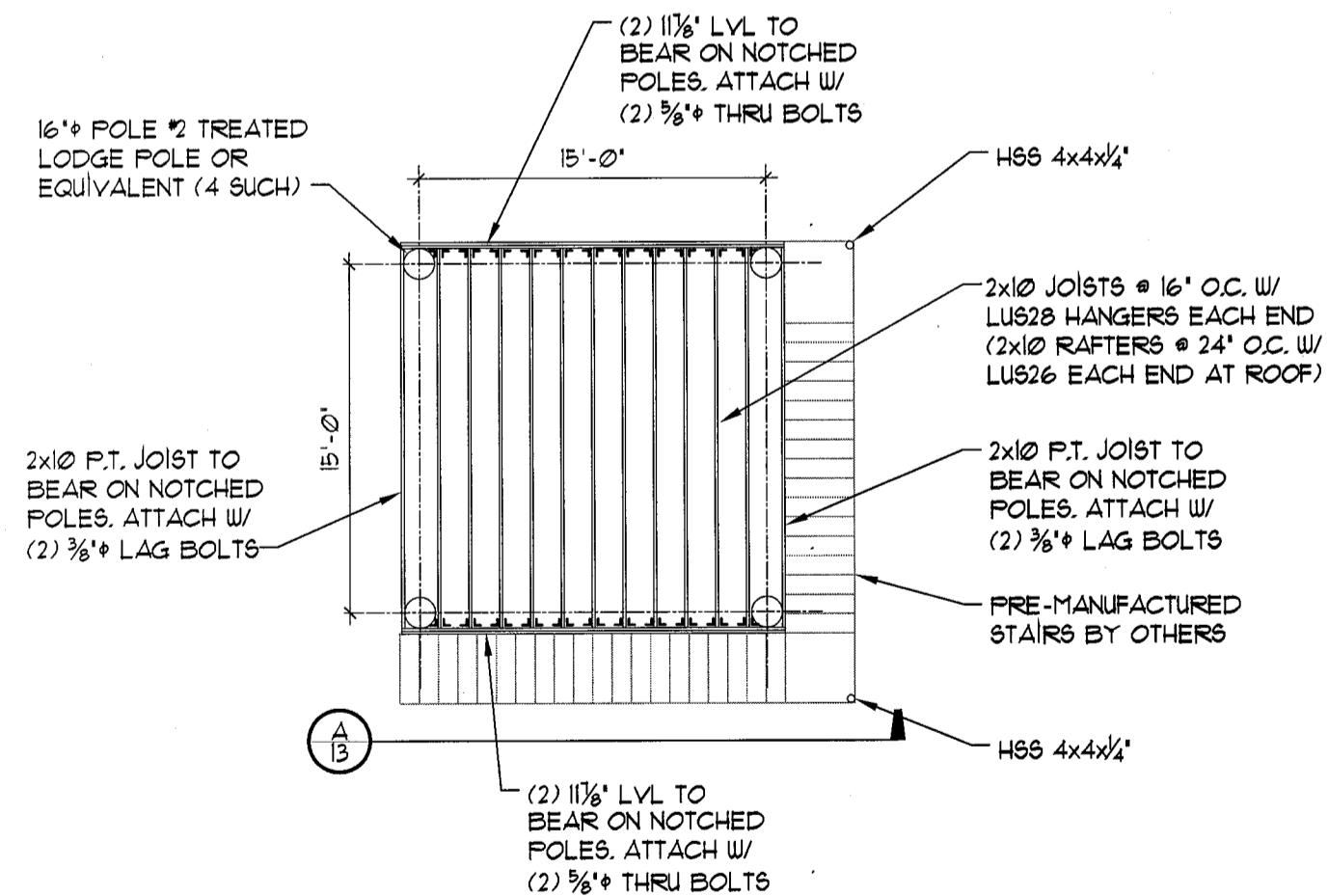
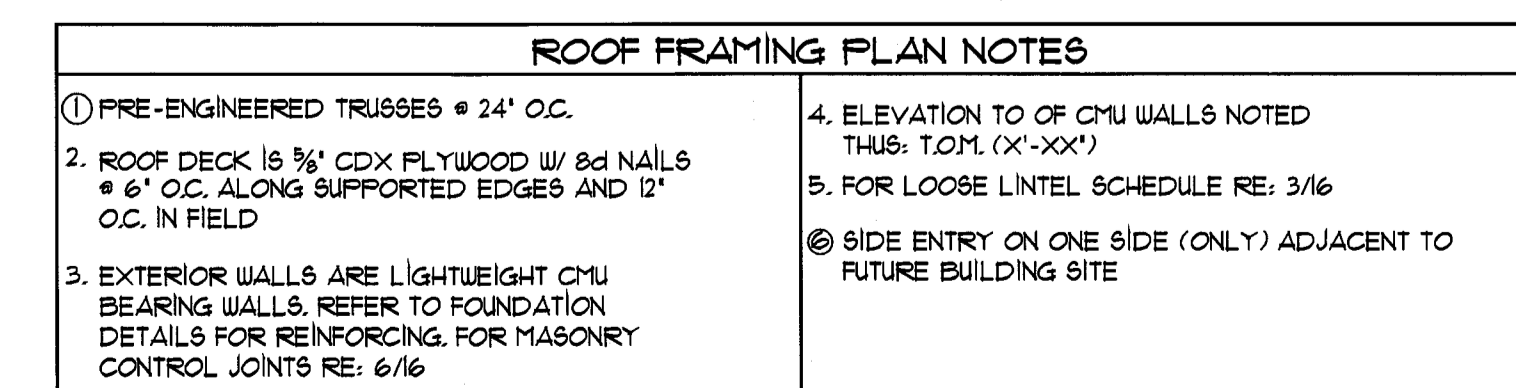
STORAGE SHED ROOF FRAMING PLAN

SCALE: 1/8" = 1'-0"



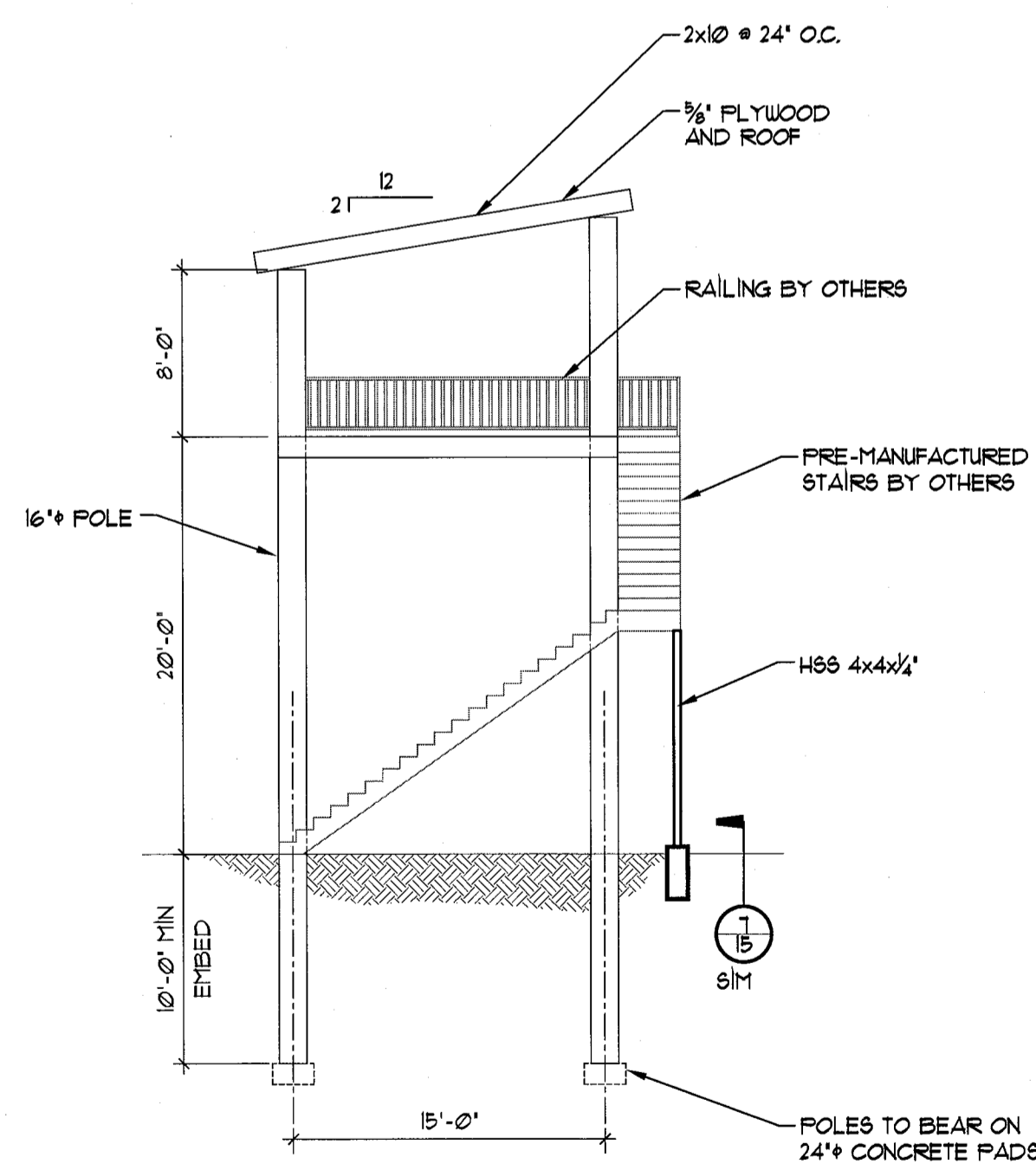
10 STATION FIRING LINE STRUCTURE ROOF FRAMING PLAN NORTH

SCALE: 1/8" = 1'-0"



RANGE CONTROL/SAFETY OBSERVATION TOWER FLOOR/ROOF FRAMING PLAN NORTH

SCALE: 1/8" = 1'-0"

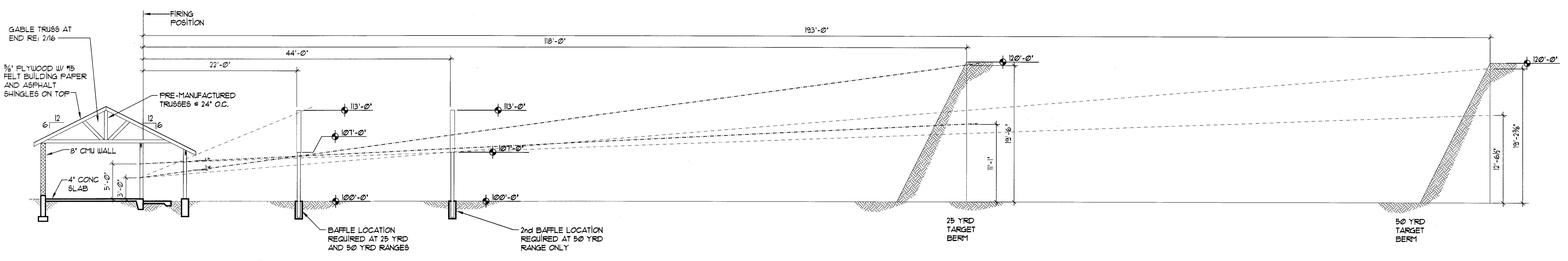
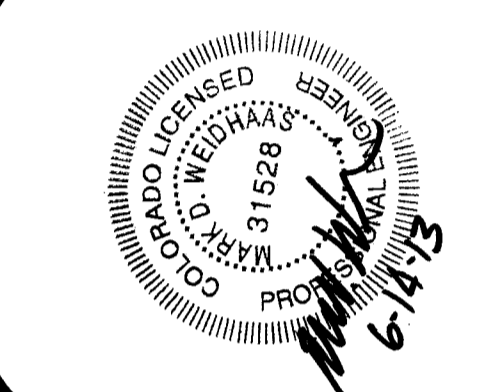


RANGE CONTROL/SAFETY OBSERVATION TOWER SECTION

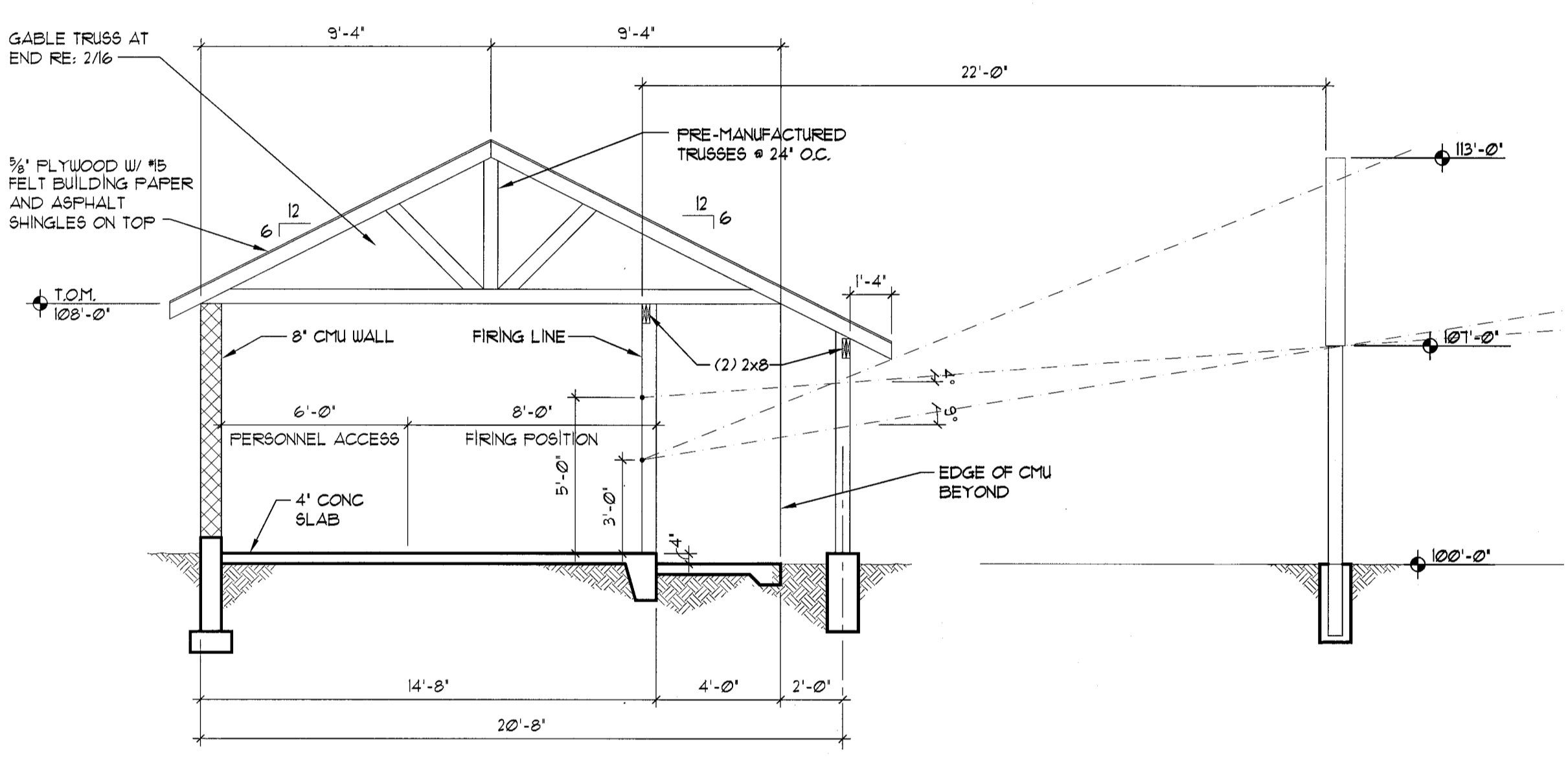
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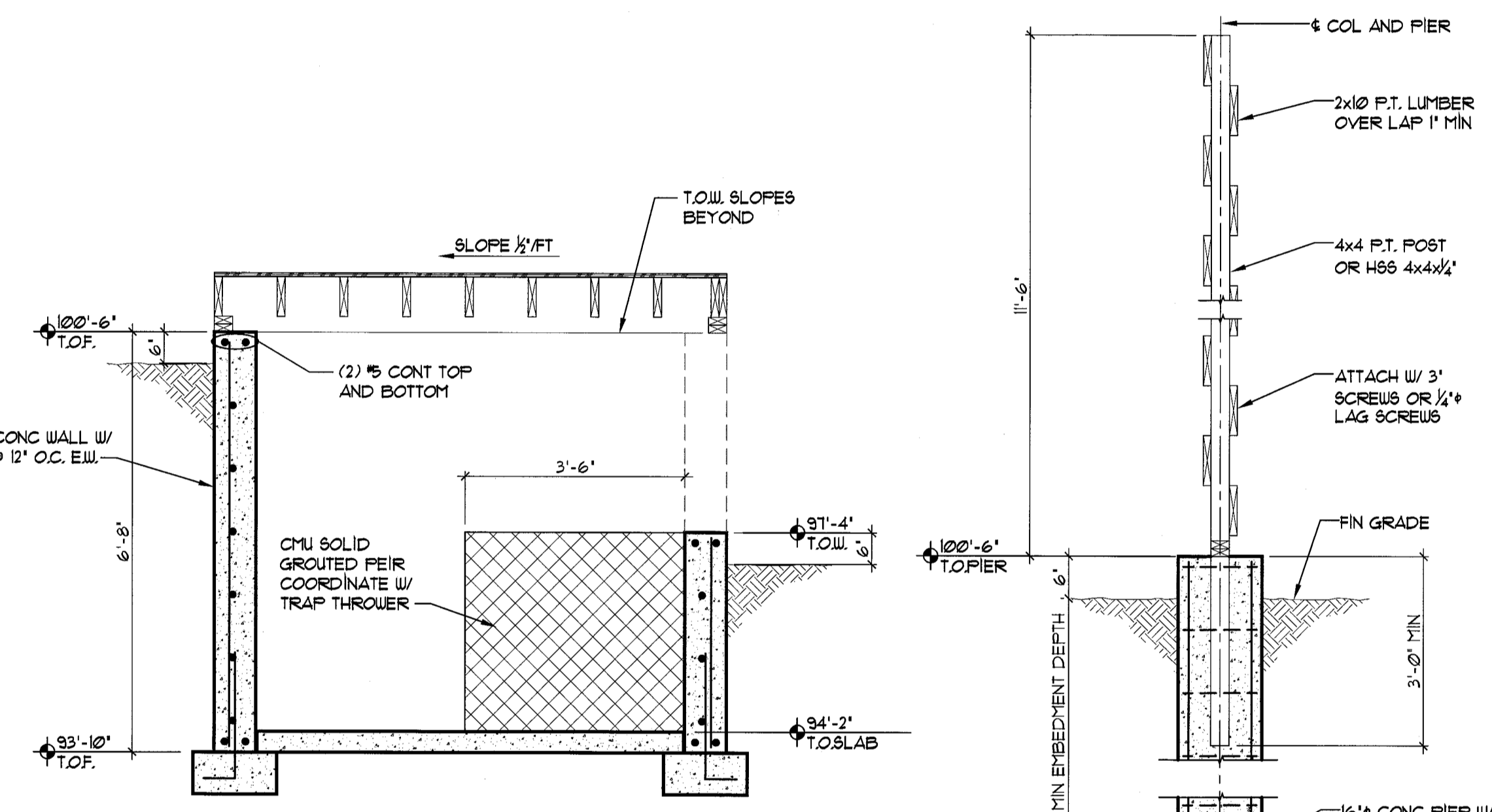
Corporate Office
2910 Austin Bluffs Parkway
Colo. Spgs., CO 80918
Voice (719) 548-0600
Fax (719) 548-0223
Castle Rock Office:
(303) 688-9475
Woodland Park Office:
(719) 687-6077
Monument Office:
(719) 488-2145
Pueblo Office:
(719) 544-7750



RANGE SECTION
SCALE: 1/8" = 1'-0"

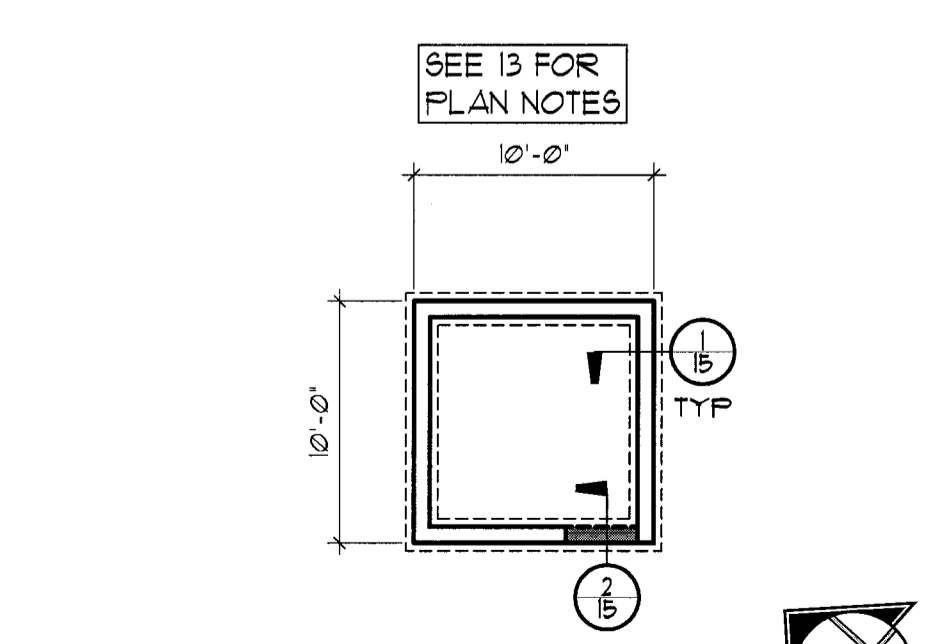


FIRING LINE STRUCTURE SECTION
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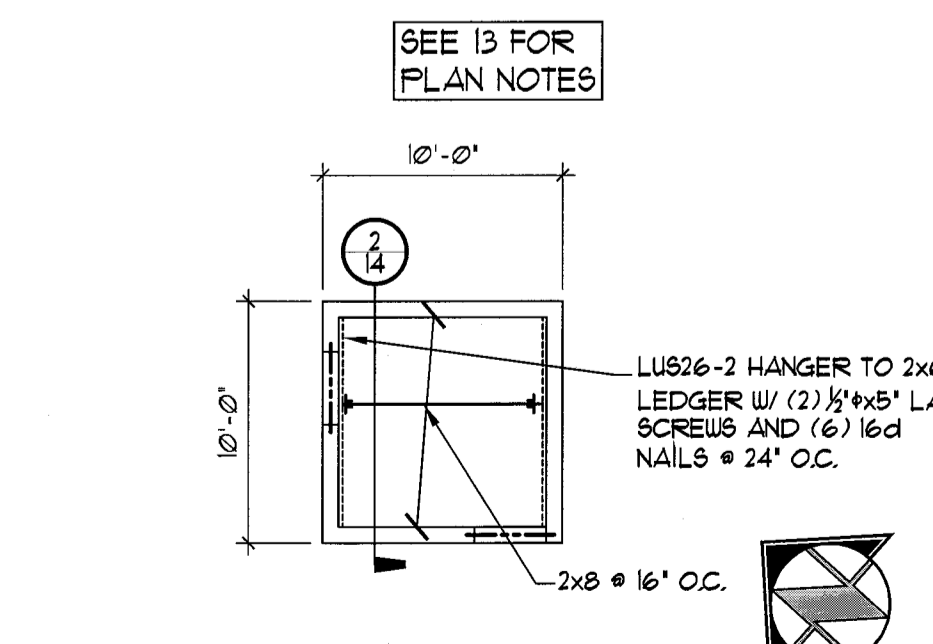


1 TRAP HOUSE SECTION
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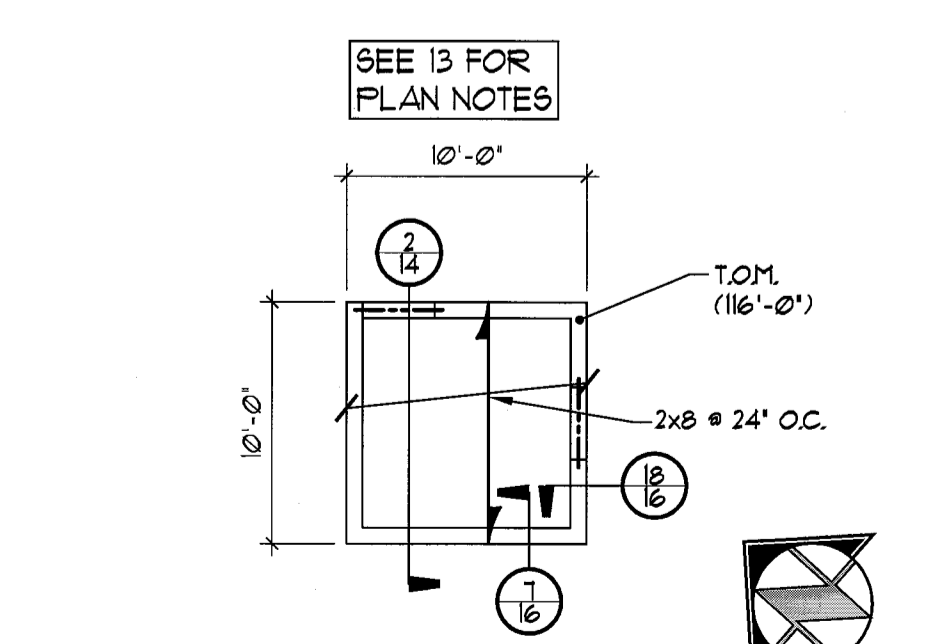
3 SKEET HOUSE WALL FOUNDATION DETAIL
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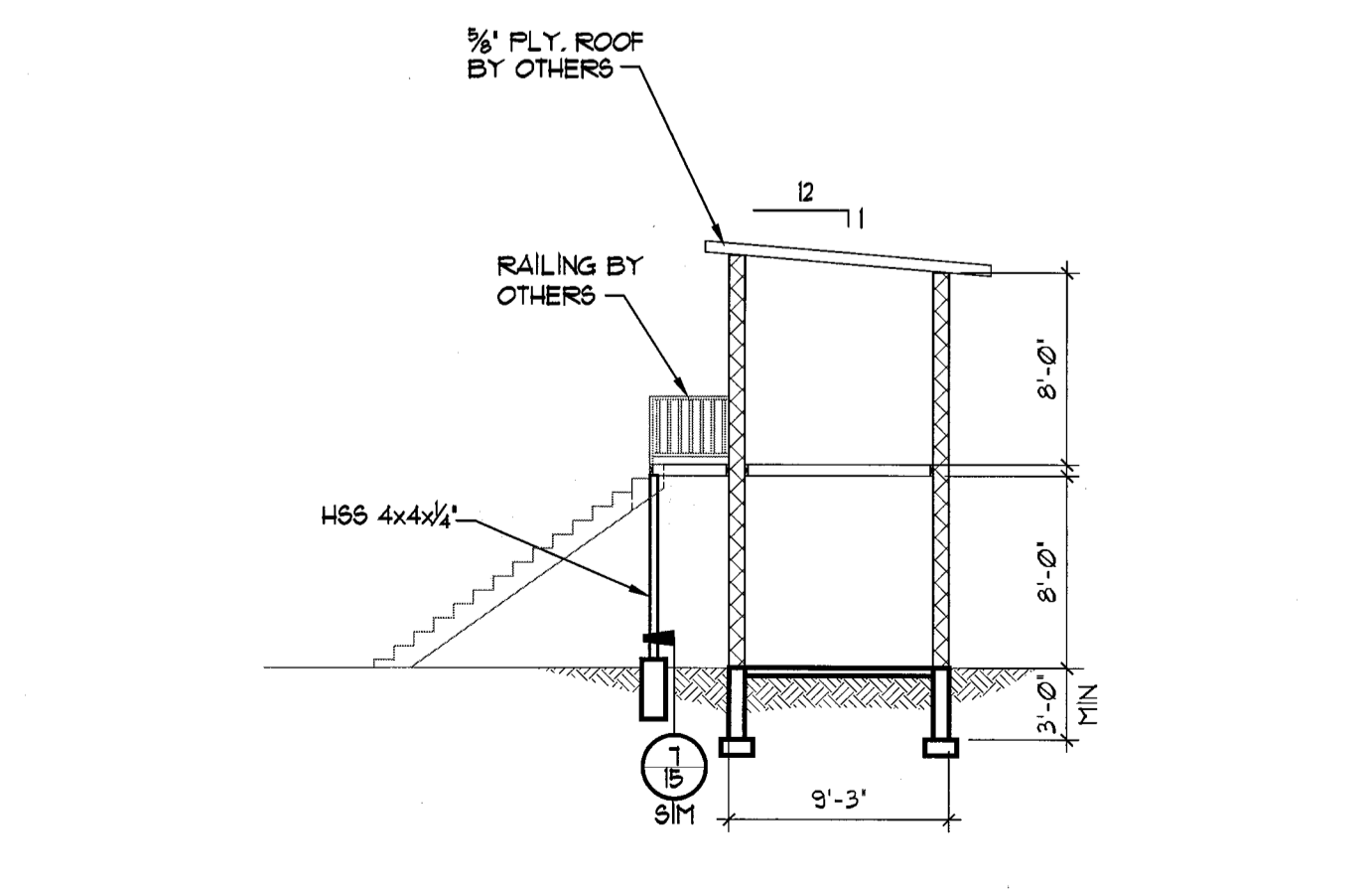
SKEET HOUSE FOUNDATION PLAN
SCALE: 1/8" = 1'-0"



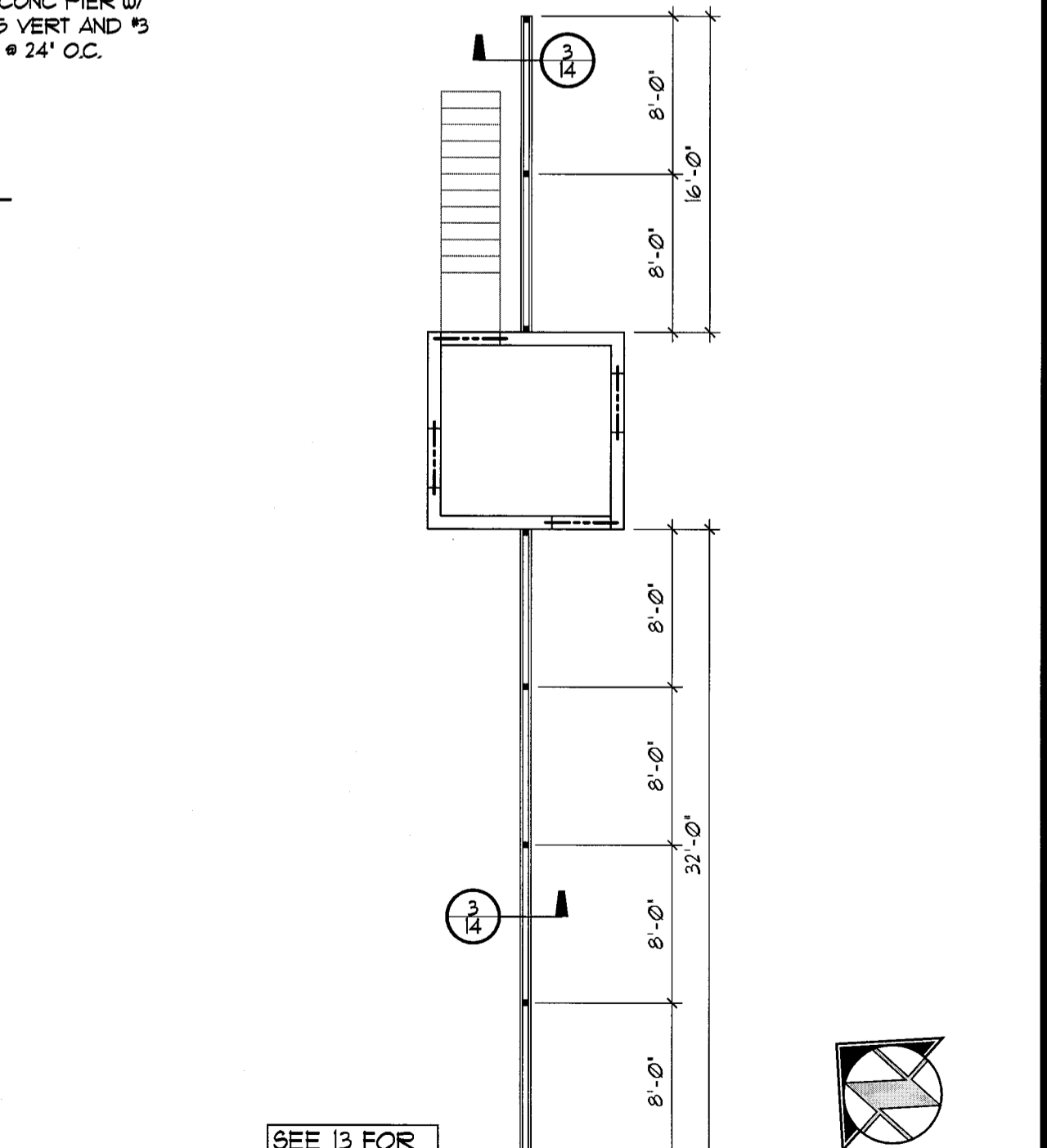
SKEET HOUSE FLOOR FRAMING PLAN
SCALE: 1/8" = 1'-0"



SKEET HOUSE ROOF FRAMING PLAN
SCALE: 1/8" = 1'-0"



2 HI-LOW SKEET HOUSE SECTION
SCALE: 1/8" = 1'-0"



SKEET HOUSE WALL PLAN VIEW
SCALE: 1/8" = 1'-0"

GOLD CAMP SHOOTING SPORTS CENTER
CR 81
TELLER COUNTY, COLORADO
TELLER COUNTY SHOOTING SOCIETY

ENGINEER:	MW
DRAWN BY:	CL
CHECKED BY:	MW
PRINTING:	DATE:
SUBMITTAL	11-05-12
REVISION:	DATE:
RESUBMIT	6-17-13

FOUNDATION FRAMING PLANS SECTIONS AND DETAILS

SHEET No.
14
OF 16
JOB No. 131963

GENERAL STRUCTURAL NOTES

1. APPLICABLE CODES:
 A. These general notes apply to all structural drawings. This project is designed in accordance with the International Building Code (IBC) 2003 Edition and the Minimum Design Loads for Buildings and Other Structures (ASCE 7-02) and the Teller County Building Code (2010 changes).
 B. All materials and workmanship shall be in accordance with applicable provisions of the codes specified above.

2. LOADS USED IN DESIGN:
 A. Roof Live Load: P_L (Balanced) 40 psf
 (and/or drift provisions ASCE 7-02, section 1.7)
 Snow Load Importance Factor, I_s 1.0
 Snow Thermal Factor, T_s 1.0
 Snow Exposure Factor, C_e 1.0
 Roof Dead Load 8 psf
 Floor Live Load 60 psf
 Floor Dead Load 10 psf
 Wind Basic Wind Speed, V 85 mph
 Exposure 'C'
 Wind Importance Factor, I_w 1.0
 Building Category I
 ii - One
 Internal Pressure Coefficient, GCF_i 0.85 psf

3. COORDINATION:
 A. **DO NOT SCALE.** Changes affecting the layout shown must be specific and clearly conveyed to RMG Engineers Group in written form as a change for inclusion into these plans. Contractor and/or client shall verify all dimensions and layout prior to construction. All dimensions on structural drawings shall be checked against architectural drawings and any discrepancies shall be brought to the attention of the Architect and Engineer immediately. Refer to mechanical, electrical and architectural drawings for openings not shown on structural drawings.
 B. Shop drawings shall be prepared by the fabricator. Copying of these construction documents for use as shop drawings will not be permitted. General contractor shall review shops prior to submitting to design team.
 C. All temporary shoring shall be the responsibility of the contractor.
 D. Design is void after two years from original date of issue, unless updated to applicable codes and practices at that time.
 E. A preconstruction meeting with personnel of RMG Engineers Group, the architect, contractor and appropriate subcontractors is strongly recommended prior to construction to discuss structural plans.

4. CONCRETE:
 A. Concrete has been designed and shall be constructed in accordance with the American Concrete Institute "Building Code Requirement Reinforced Concrete and Specifications for Structural Concrete for Buildings" (ACI 308 and ACI 309) latest editions. Section 13 "Inspection" of ACI 308 is deleted in its entirety, see "Field Observations" paragraph. All concrete shall be of stone aggregate, unless noted otherwise.
 B. **Concrete Tests:**
 See specifications for any additional durability requirements.
 (1) 4000 psi minimum compressive strength at age of 28 days.
 Type I/II Cement, minimum of 540 pounds per cubic yard.
 Fly ash not allowed.
 (2) $3/4"$ maximum aggregate size.
 (3) Maximum air: 5%
 (4) $4"$ (8" with superplasticizer) maximum slump.
 (5) Water reducing agent: 1.0%
 Use in accordance with manufacturer's recommendations.
 (6) 1000 psi For footings, grade beams, and miscellaneous concrete.
 (7) 3000 psi minimum compressive strength at age of 28 days.
 Type I/II Cement, minimum of 410 pounds per cubic yard.
 (8) $3/4"$ maximum aggregate size.
 (9) 1% Entrained Air.
 (10) $4"$ (8" with superplasticizer) maximum slump.

C. Reinforcing is to be new billet steel ASTM A63, Grade 60, except ties and bars to be used shall be Grade 40. Provide not less than (2) #5 around all sides of all openings in concrete and extend 2'-0" past edges of openings. No splices of reinforcement are permitted except as detailed or authorized by structural engineer. Where permitted, use contact lap splices (36" lap diameter minimum). Lapped steel fabric (WJF) shall be in accordance with ASTM A888. Lap (1) full mesh minimum at splices. No welding of reinforcement permitted unless detailed.
 D. Placing of Reinforcement: Provide chairs, bolsters, additional reinforcement, and accessories necessary to support reinforcement at position shown on drawings. Support of reinforcement on form ties, wood, brick, brickbat or other unacceptable material will not be permitted.
 E. Reinforcement shall be placed so that the following minimum concrete protection is provided, unless noted otherwise:
 (1) Concrete surfaces poured against ground 3" Clear
 (2) Formed surfaces exposed to ground or weather:
 a) Bars #5 and larger 2" Clear
 b) Bars #3 and smaller 1 1/2" Clear
 (3) Slabs 1 1/2" Clear (unco.)
 (4) Concrete not exposed to earth or weather 3/4"
 (5) Beams, Columns, Ties, Straps or spirals around:
 a) Primary reinforcement 1 1/2"
 b) Secondary reinforcement, or primary reinforcement with no ties, straps or spirals 1 1/2"
 F. Foundation elements below grade shall have backfill placed equally on both sides until the required level is reached. Walls shall be appropriately shored when backfill is placed on one side only.
 G. Additional (2) #5 bars (one each face) with a 2'-0" projection shall be placed diagonally across the corners of all openings and vertical steps in walls unless otherwise detailed on plans.
 H. The contractor is responsible for determining when it is safe to remove forms and/or shoring. Forms and shoring must not be removed until the walls are strong enough to carry their own weight and any anticipated superimposed loads. For foundation walls, this typically requires at least 12 hours of cumulative curing time at a temperature of 50°F or more. Concrete must be adequately covered during cold periods to maintain its surface temperature. Due to varying weather conditions, alternative curing processes, and the use of Type III cement, RMG Engineers Group suggests forms remain in place a minimum of 3 days to ensure the performance specification has been met. When forms are stripped there must be no excessive deflection or distortion or discoloration and no evidence of damage to the concrete. Adequate thermal protection of the concrete shall be continued after stripping for a cumulative period of 48 hours at 50°F or more, after the initial pour. See applicable notes for specifications on when to backfill foundation walls.

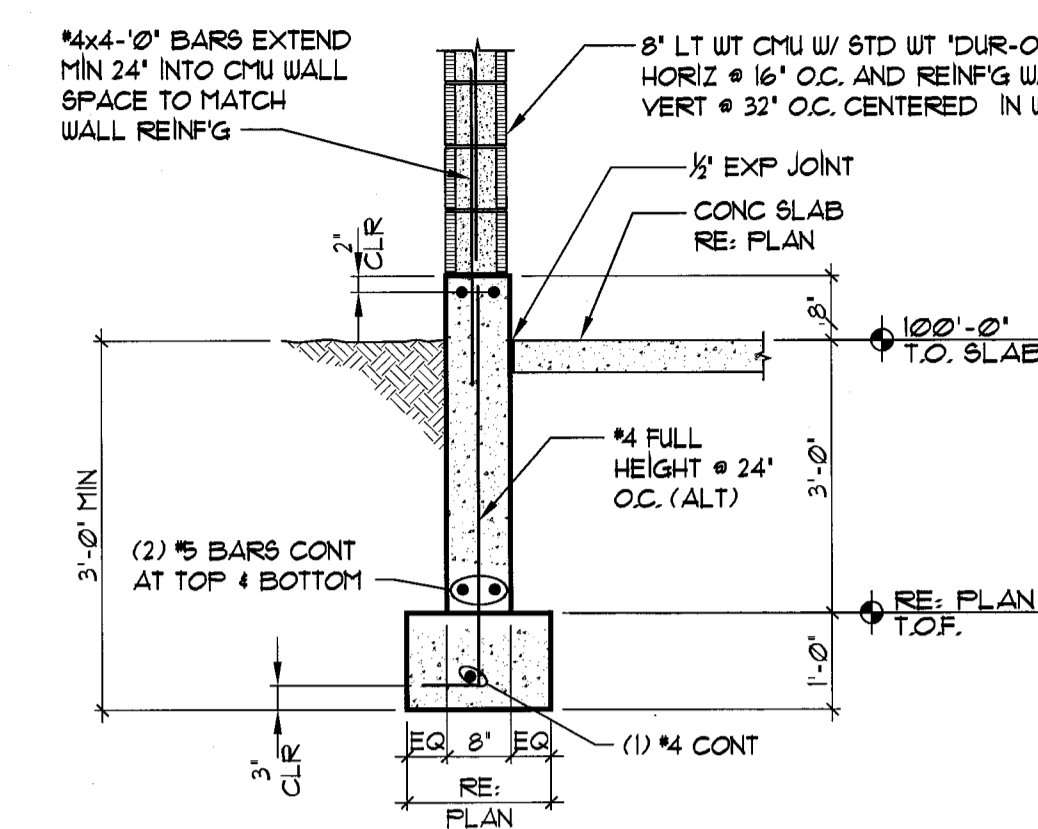
I. **Field Quality Control:**
 (1) Reference standards: ACI 301 Chapters 16 and 17, latest edition.
 (2) Slump tests: The general contractor shall provide necessary equipment and shall make tests in conformity with ASTM C143. The contractor shall make slump tests on the first truck of each pour and as often as deemed necessary by the contractor to maintain the required slump tests when directed by the Architect or Engineer.
 (3) Control tests:
 a. Control tests of concrete work shall be made on every 50 cubic yards or fraction thereof of concrete placed and, in any case, minimum of once during each day's pour.
 b. Each test shall consist of four standard 6" test cylinders cast and cured in accordance with ASTM C31 and ASTM C172.
 c. Sample concrete at point of placement.
 d. One cylinder shall be broken at end of seven days after placing, two cylinders shall be broken at end of 28 days after placing, and remaining cylinder shall be stored until its disposition is determined by Architect.
 e. In general, remaining cylinder will be broken only when previous test reports indicated unsatisfactory results.
 f. Tests on remaining cylinder shall be at expense of the contractor.
 g. Architect and/or Engineer reserves right to stop future concrete work when seven or 28 day tests indicate unsatisfactory results until, in the opinion of the Architect and/or the Engineer of Record, proper corrective measures have been taken to insure quality concrete in future work and corrections deemed necessary have been made.
 h. Tests shall be made at the control tests are taken and so stated in reports to determine slump, air content, unit weight and temperature of concrete.
 i. All tests shall be made in accordance with ASTM C188 or ASTM C231.
 (4) Slab tolerances: Maintain surface flatness with maximum variation of 1/8 inch in 20 feet.

5. WOOD:
 A. Framing lumber shall be Hem Fir (unless noted otherwise) and as follows or better:
 2x4 studs 2nd Grade
 2x6 or larger studs 3rd Grade
 Plates 3rd Grade
 Joists and Rafters 4th Grade
 2x and 4x Beams 4th Grade
 6x or larger Beams 4th Grade
 Girders 24" V4 DDF unless noted otherwise
 Posts 4th Grade Post and Timber
 B. All wood construction shall be in conformance with the provisions of "The National Design Specification for Wood Construction", latest edition.
 C. Laminated Veneer Lumber (LVL) and precast joists shall be manufactured by "TruLam" or equivalent or shall meet APA Performance Standards, and installed per manufacturer's specifications. Supplier shall furnish shop drawings showing all joints, bridging, blocking and miscellaneous accessories for review by the structural engineer prior to fabrication.
 D. Where not otherwise shown on plans, all nailing or screwing shall be as indicated in the Building Code. All sheathing must be nailed. Adhesives SHALL NOT be used in place of nailing.
 E. Metal connectors to be provided by Simpson Strong-Tie or equivalent.
 F. APA rated OSB may be used in lieu of plywood with prior approval from Engineer of Record.
 G. Wood roof trusses shall be designed by others unless noted otherwise. Calculated live load deflection of trusses shall not exceed 1/360 for floors and 1/240 for roof of the overall span length. The truss supplier shall provide shop drawings and calculations prepared and stamped by a structural engineer registered in the state of Colorado for review by the Engineer of Record to verify they conform to the requirements of the basic structure. These shop drawings shall show the locations of all trusses, connection plate sizes & capacity and the size & grade of lumber to be used. Truss fabrication shall not proceed until completion of shop drawing review by the Engineer of Record. Truss manufacturer or contractor shall provide blocking at bearing locations and bridging/lateral bracing as required for truss stability.
 H. Site fabricated trusses are to be adequately shored and installed by qualified personnel. Appropriate bracing shall be in place at all times.
 I. RMG Engineers Group is not responsible for the construction sequence of site built trusses.
 J. The contractor shall not cut, notch or otherwise modify joists without the written consent of the Engineer of Record.

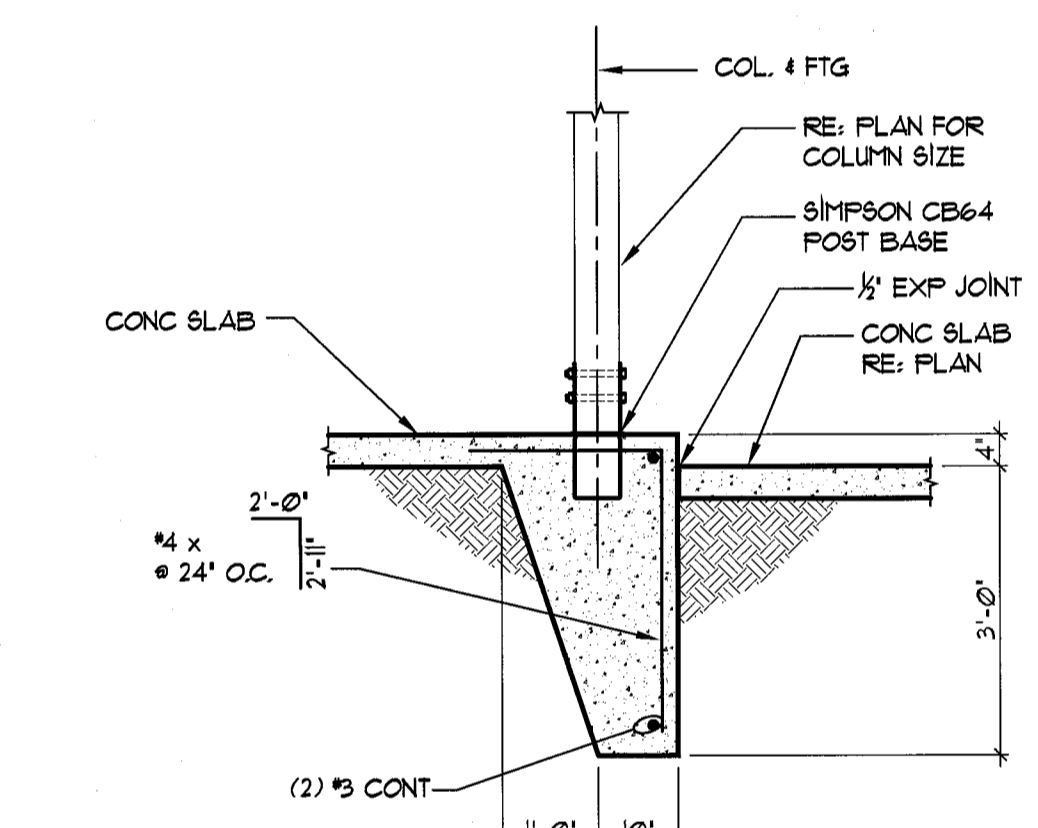
6. MASONRY:
 A. Concrete block walls shall have a minimum ultimate compressive strength (f'm) of 1500 psi. Concrete masonry units (CMU) shall conform to ASTM C90 grade-N made with light weight aggregates. Mortar shall conform to ASTM C778 Type-S. Grout shall conform to ASTM C416 with a minimum compressive strength of 2000 psi in 28 days.
 B. Vertical reinforcement shall conform to ASTM A63, Grade 60. Where splices are required, provide minimum 40" lap diameter lap. Horizontal joint reinforcing shall be standard weight "Dur-O-Wall" or equivalent at 16" o.c. maximum. Provide prefabricated corner and tee sections at wall corners and intersections.
 C. Provide a minimum #5 vertical bar, or as noted on plan, full height at each door jamb, window jamb, mechanical opening, control joint, or other masonry discontinuity. Locate reinforcing so as to be continuous without interfering with lintel bearing.
 D. Special inspection and testing shall be required in accordance with section 104.4 of the 2003 International Building Code.
 E. Provide cleanouts at the bottom of groutout cells and use High-Flow grouting procedures where possible. Mechanical vibrators shall be used to consolidate grout and reconsolidate grout 15 minutes after the initial consolidation. Any masonry in contact with earth shall be grouted full and back-parged as required.

7. SPREAD FOOTING FOUNDATIONS:
 A. The foundation design has been completed in accordance with pertinent standards, recommended design soil parameters, accepted engineering design procedures, and is based on the best information available at the time of completion. The design is intended to minimize differential movement resulting from the heaving of expansive soil or settling of subsurface soils. It must be recognized that foundation components will undergo movement. It shall be the responsibility of the contractor and/or present owner to inform any subsequent owners of the soil condition and advised to maintain good practices in the future with regard to surface and subsurface drainage, framing of partitions above floor slabs, and finish work above the floor slabs, etc.
 B. Foundation design is based on soil report No. 032280 prepared by RMG Engineers Group dated 10-19-12. The Contractor shall thoroughly review and understand all pertinent construction aspects of this report before beginning any work.
 C. Foundation Design parameters include allowable bearing pressure of 2500 psf with no minimum dead load requirement and with soil preparation per soil report.
 D. A representative of the Geotechnical engineer shall observe the open excavation to determine that the soil type and conditions are consistent with design criteria of the soil report. If the soil properties are found to be different from the criteria, the foundation engineer shall be promptly notified so that the foundation design may be reviewed.
 E. The contractor shall be responsible to coordinate the location of mechanical openings, floor drains, inserts, depressions, buried cables and utilities, etc. with architectural, civil, mechanical and electrical drawings.
 F. Mechanically compact all interior and exterior backfill per Geotechnical engineer's recommendations. It will also be necessary to adjust and maintain the grade immediately against foundations periodically to avoid the creation of a water trap as the backfill settles over time.
 G. Slope backfill away from the building a minimum of 10% for the first 10 feet (2% at paved areas) unless a more stringent requirement is specified by the Geotechnical engineer. Carry roof drains across the backfilled areas. Do not allow water to stand or pond near the building. Do not flood the backfill.
 H. Contact Geotechnical engineer for proper preparation of subgrade for placement of floor slabs.
 I. Floor slabs have a high probability of moving vertically. Floor slabs shall be separated from all structural portions of building with an expansion joint of minimum 1/2" thick Styrofoam or other approved joint material. A gap in non-bearing partitions, the non-rigid connections with the stationary construction and non-rigid construction of door jambs may be required by the Geotechnical engineer. If required, these items may also require reconstruction over the life of the structure to maintain the independent vertical movement of the floor slabs.
 J. Unless a specific top of wall connection is shown, foundation wall stability is not dependent on floor framing for lateral support. A stud wall (cripple wall) or maximum of (3) 2x4 or (4) 2x6 plates (in addition to embedded wall) are allowed along the top of the foundation wall (including garden level walls) unless noted otherwise. Walls having backfill on both the interior and exterior faces should have backfill on either side brought up approximately together. Otherwise, where possible, no exterior backfill should be placed until the floor slab is in place or the bottom of the foundation wall is otherwise properly braced. Top of wall must also be braced if backfill is placed within 14 days of concrete pour.

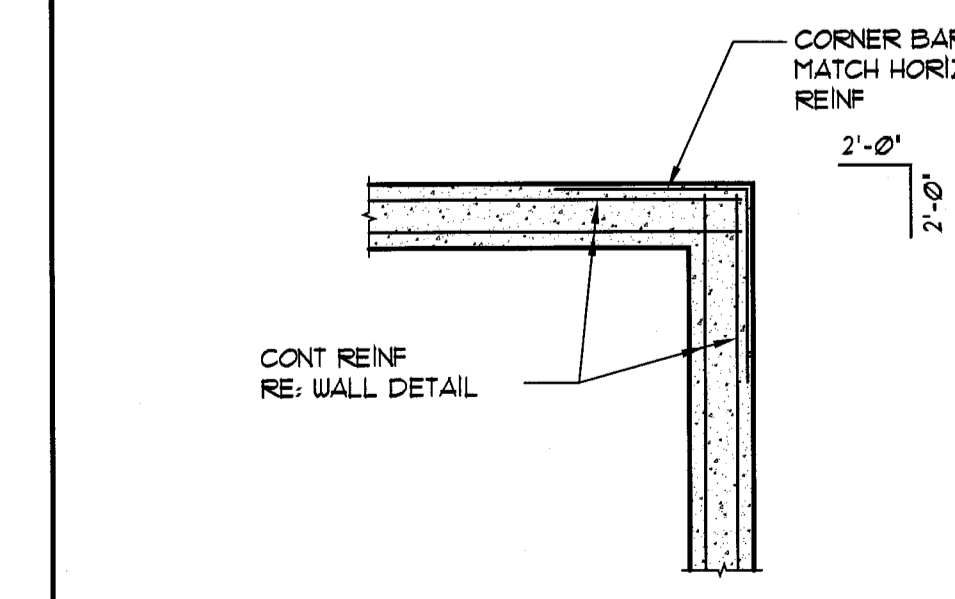
8. FIELD OBSERVATIONS:
 A. The Contractor shall inform the Engineer of Record at least 24 hours prior to casting any concrete so as to allow the Engineer of Record the opportunity to review the placement of reinforcing and/or embedded items. Contact RMG Engineers Group: (719) 548-0600.



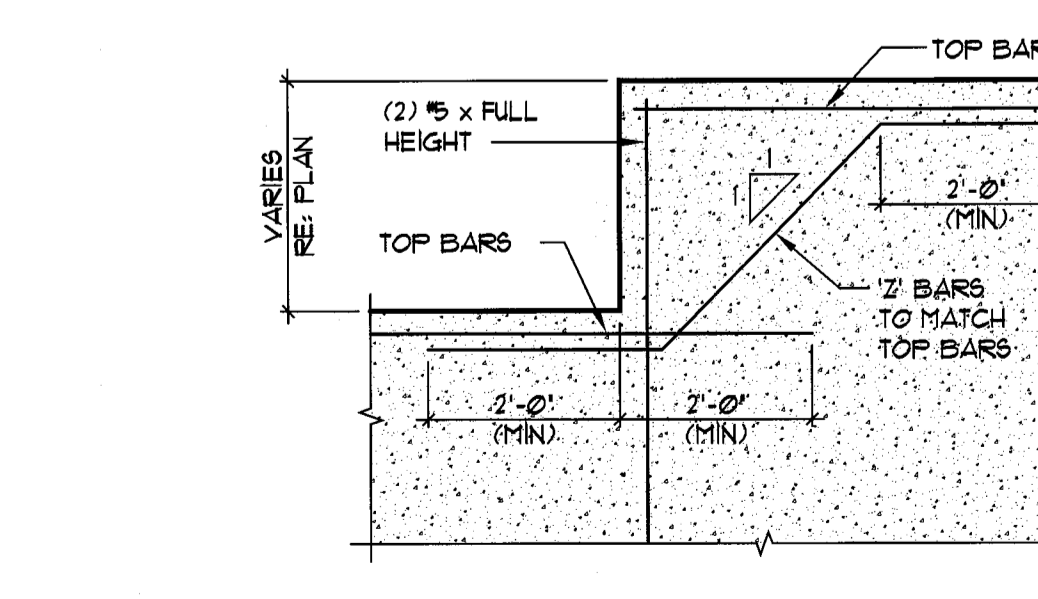
1 TYPICAL GRADE BEAM DETAIL
 SCALE: 1/2" = 1'-0"



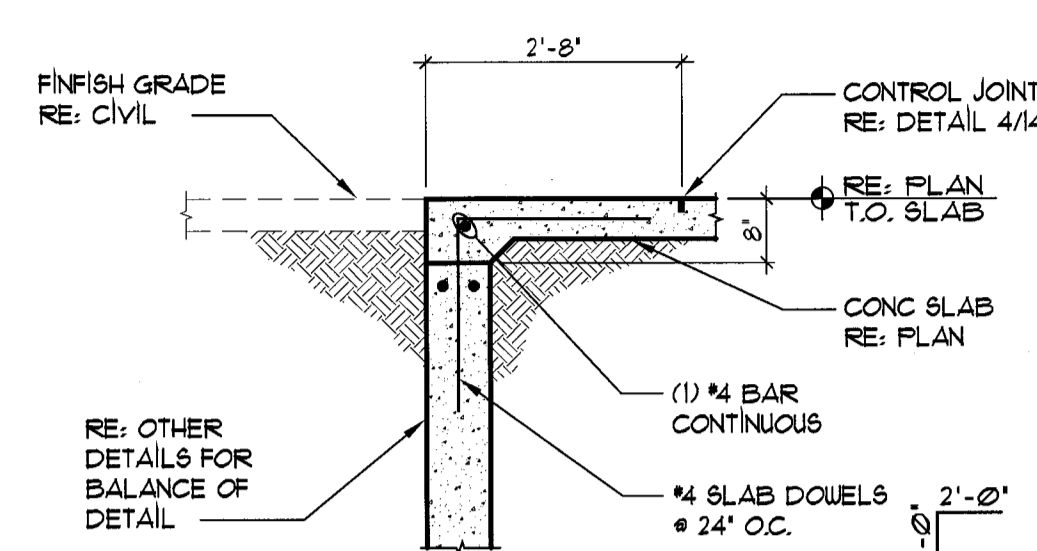
3 FOUNDATION DETAIL
 SCALE: 1/2" = 1'-0"



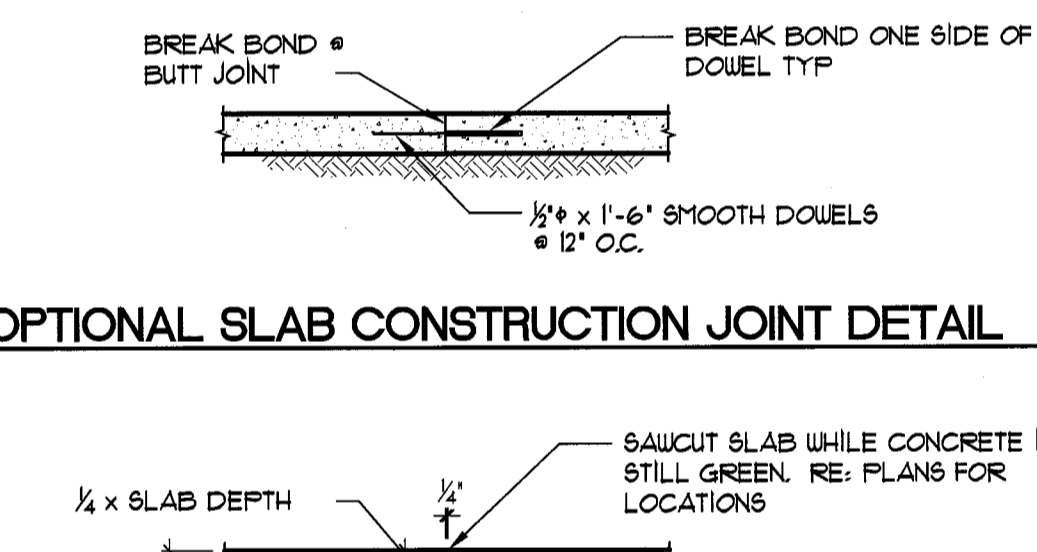
5 TYPICAL CORNER WALL REINFG DETAIL
 SCALE: 1/2" = 1'-0"



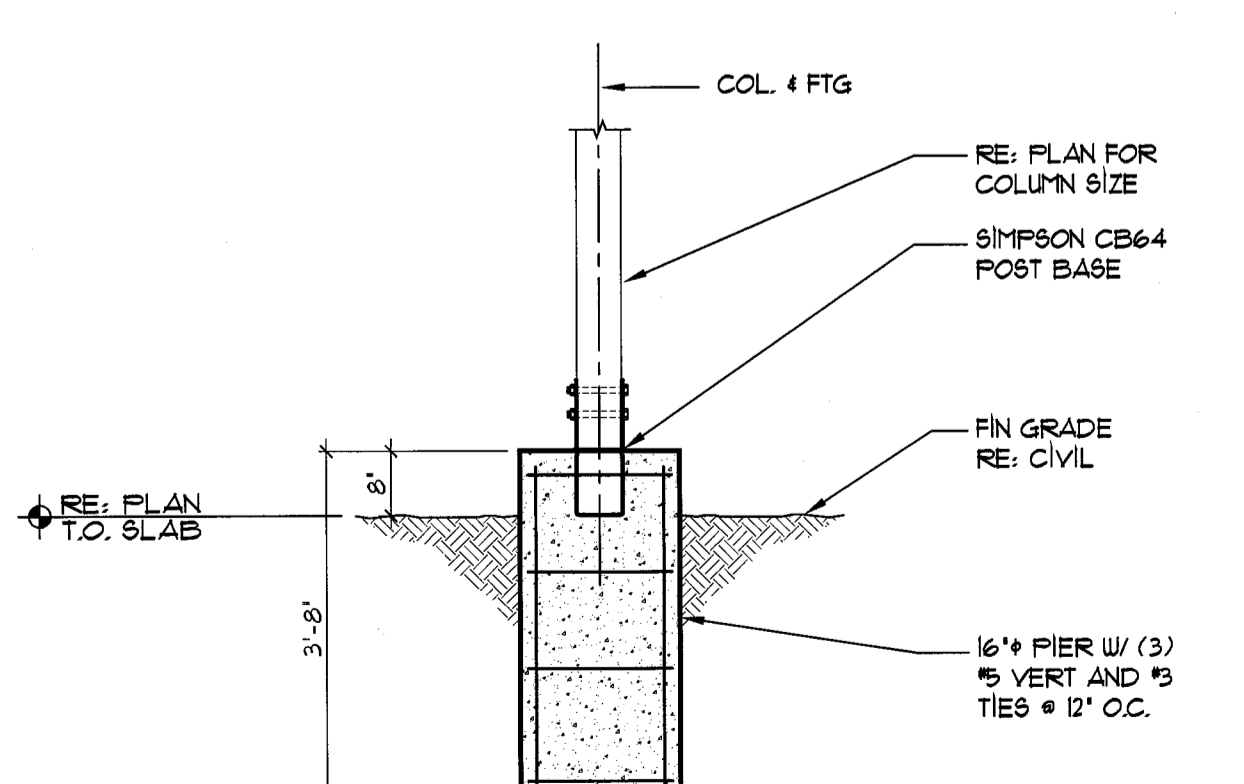
6 TYPICAL TOP OF WALL STEP
 SCALE: 1/2" = 1'-0"



2 FOUNDATION DETAIL
 SCALE: 1/2" = 1'-0"



4 TYPICAL SLAB CONSTRUCTION JOINT DETAIL
 SCALE: 1/2" = 1'-0"



7 COLUMN FOOTING DETAIL
 SCALE: 1/2" = 1'-0"

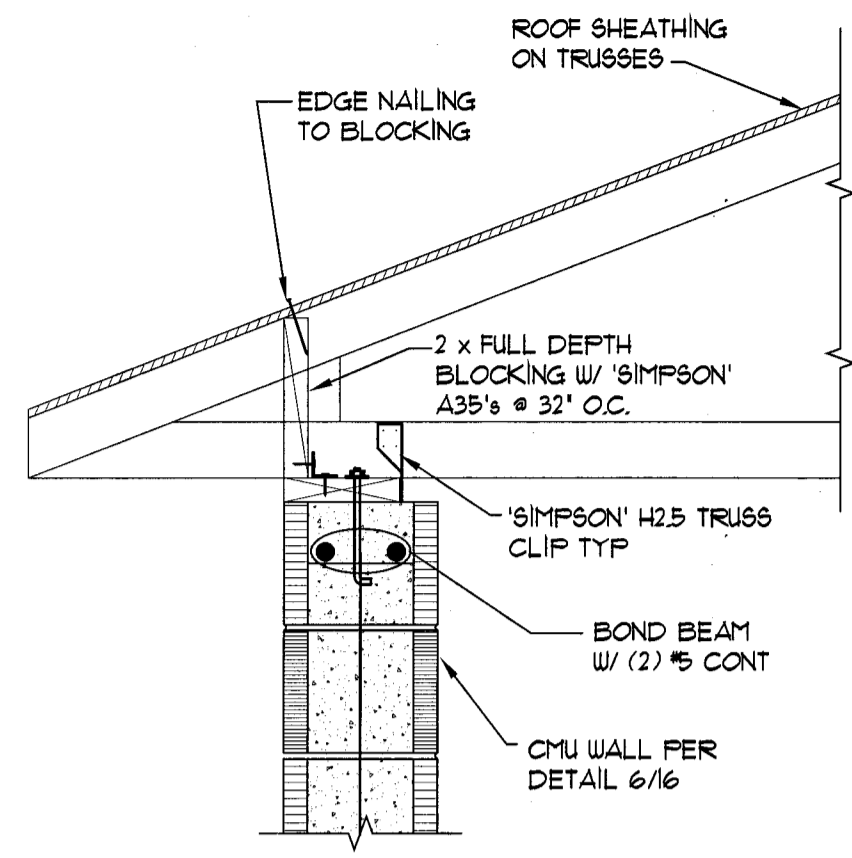
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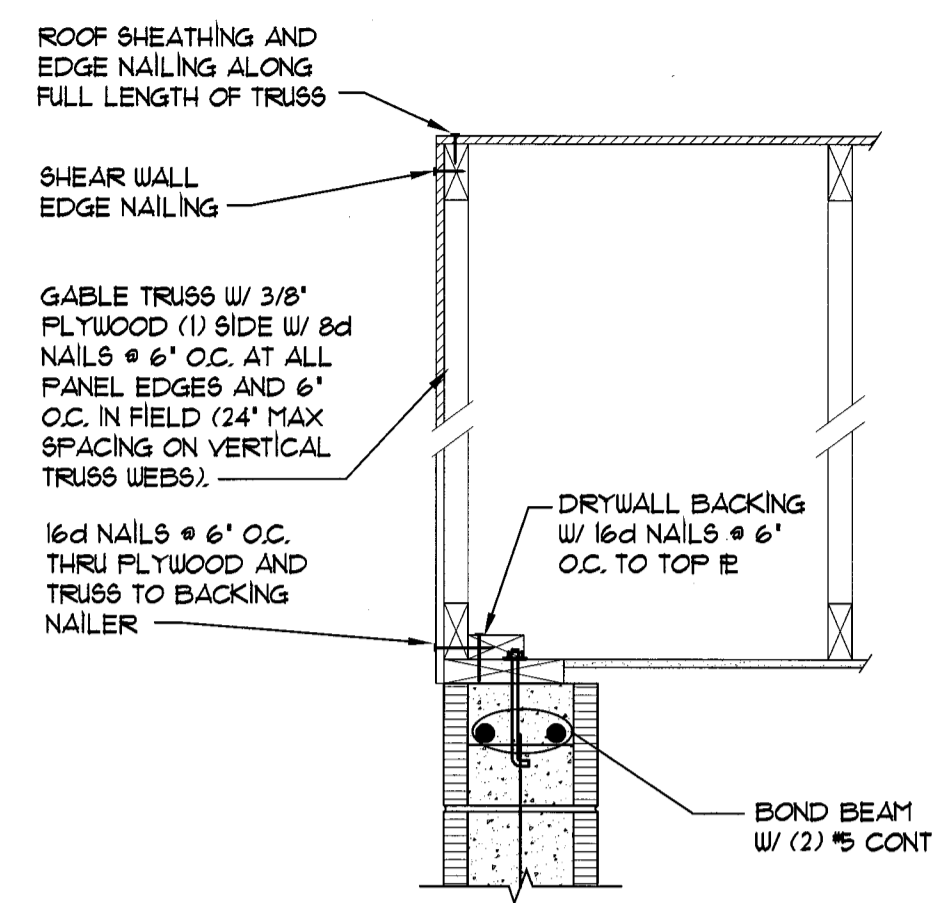
GOLD CAMP SHOOTING SPORTS CENTER
 CR 81
 TELLER COUNTY, COLORADO
 TELLER COUNTY SHOOTING SOCIETY

ENGINEER:	MW
DRAWN BY:	CL
CHECKED BY:	MW
PRINTING:	DATE:
SUBMITTAL:	11-05-12
REVISION:	DATE:
RESUBMIT:	6-17-13

GENERAL NOTES AND DETAILS
 SHEET No.
15 OF 16
 JOB No. 131963
 SITE SPECIFIC William Gunn & Associates



1 ROOF FRAMING DETAIL
SCALE: 1" = 1'-0"

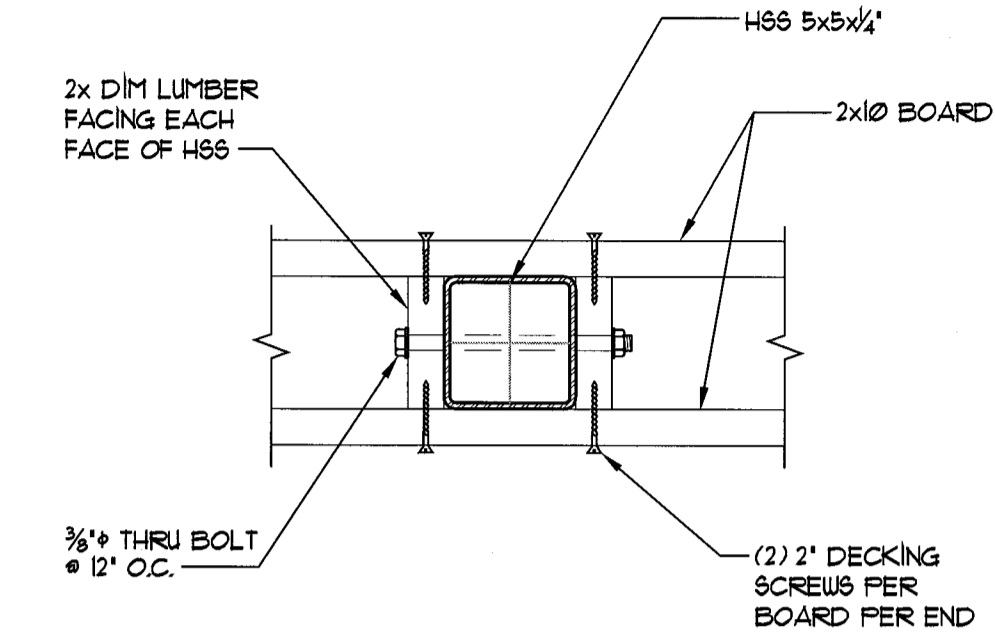


2 ROOF FRAMING DETAIL
SCALE: 1" = 1'-0"

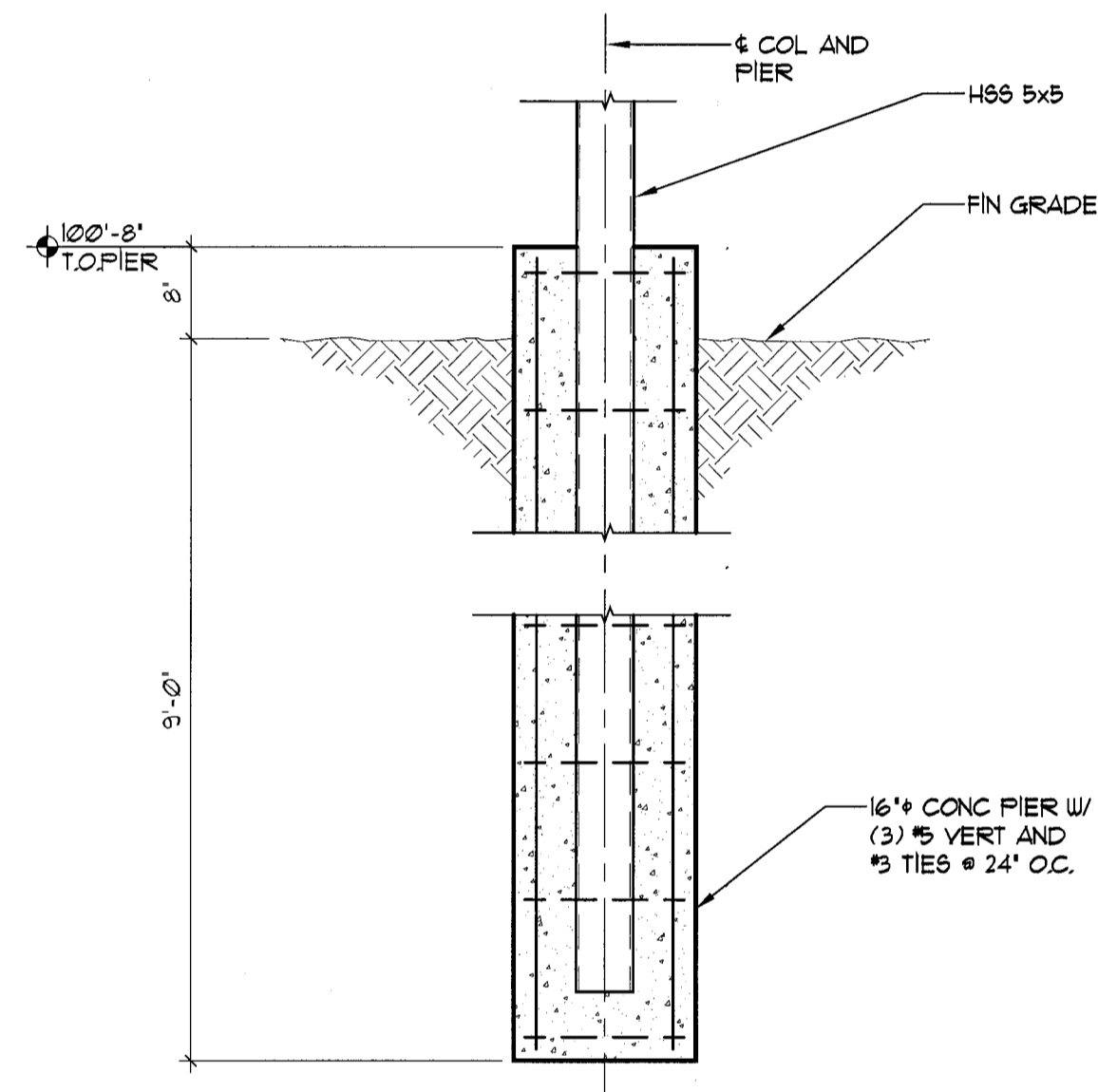
LOOSE LINTEL SCHEDULE AT EXISTING BUILDING		
SPAN	LINTEL SIZE	BRG
0' to 1'-4"	1/4" PLATE	4"
1'-4" to 3'-6"	L3 1/2" x 3" x 1/4" (LL.V)	4"
3'-6" to 4'-6"	L3 1/2" x 3 1/2" x 1/4" (LL.V)	6"
4'-6" to 5'-6"	L4 x 3 1/2" x 1/4" (LL.V)	6"
5'-6" to 7'-0"	L5 x 3 1/2" x 3/8" (LL.V)	6"
7'-0" to 8'-6"	L6 x 3 1/2" x 3/8" (LL.V)	8"
8'-6" to 11'-0"	L7 x 4" x 3/8" (LL.V)	8"

NOTES:
 1. (1) ANGLE FOR EACH 4" W/ THE OF MASONRY
 2. REFER TO ARCHITECTURAL & MECHANICAL DRAWINGS FOR LOCATION & SIZE OF OPENINGS
 3. BEARING DISTANCES SHOWN ABOVE ARE REQUIRED MINIMUMS AT EACH END OF LINTEL
 4. USE ABOVE SCHEDULE UNLESS NOTED OTHERWISE ON PLANS
 5. GENERAL CONTRACTOR TO COORDINATE HEAD HEIGHTS AND INSTALLATION OF LOOSE LINTELS WITH ARCHITECTURAL DEMO PLANS AND EXISTING CONDITIONS. STRUCTURAL FRAMING STRUCTURE ABOVE IS TO BEAR AT LEAST 1/2 LINTEL SPAN (OR MORE) ABOVE THE NEW LINTEL. IF FIELD VERIFIED TO BE LESS, OR IF MIN LINTEL BEARING CANNOT BE ACHIEVED, CONTACT RMG ENGINEERS GROUP TO DETERMINE IF ADDITIONAL STRUCTURAL SUPPORT IS REQUIRED

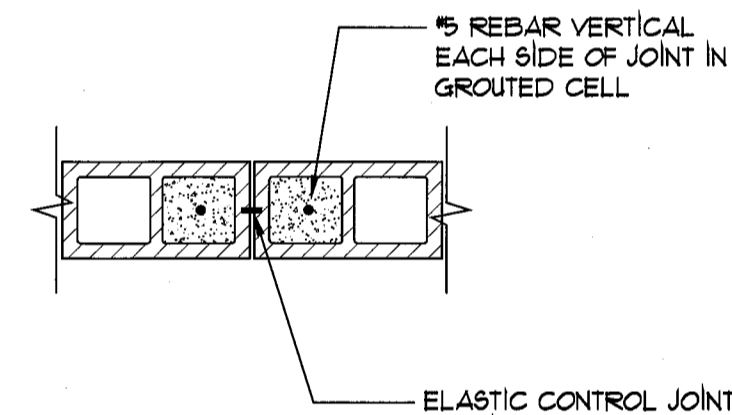
3 LOOSE LINTEL SCHEDULE
SCALE: 1" = 1'-0"



4 BAFFLE DETAIL
SCALE: 1 1/2" = 1'-0"

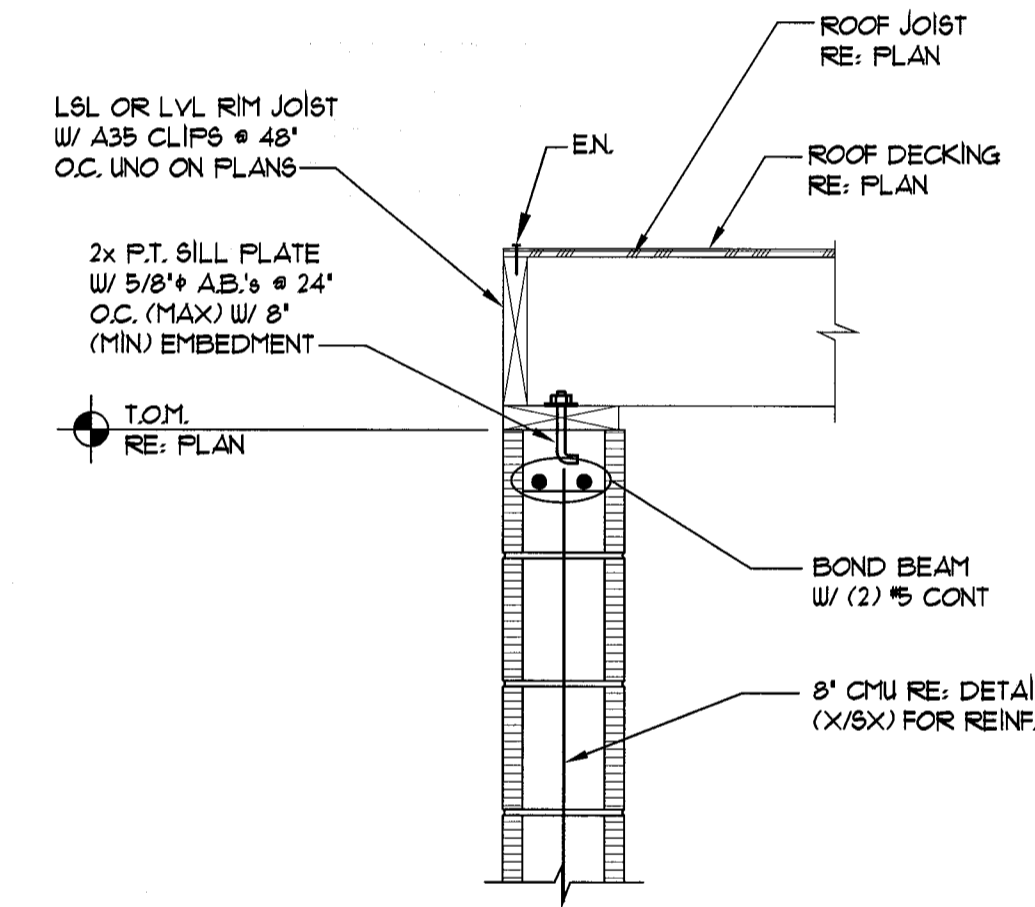


5 BAFFLE POLE FOUNDATION DETAIL
SCALE: 3/4" = 1'-0"

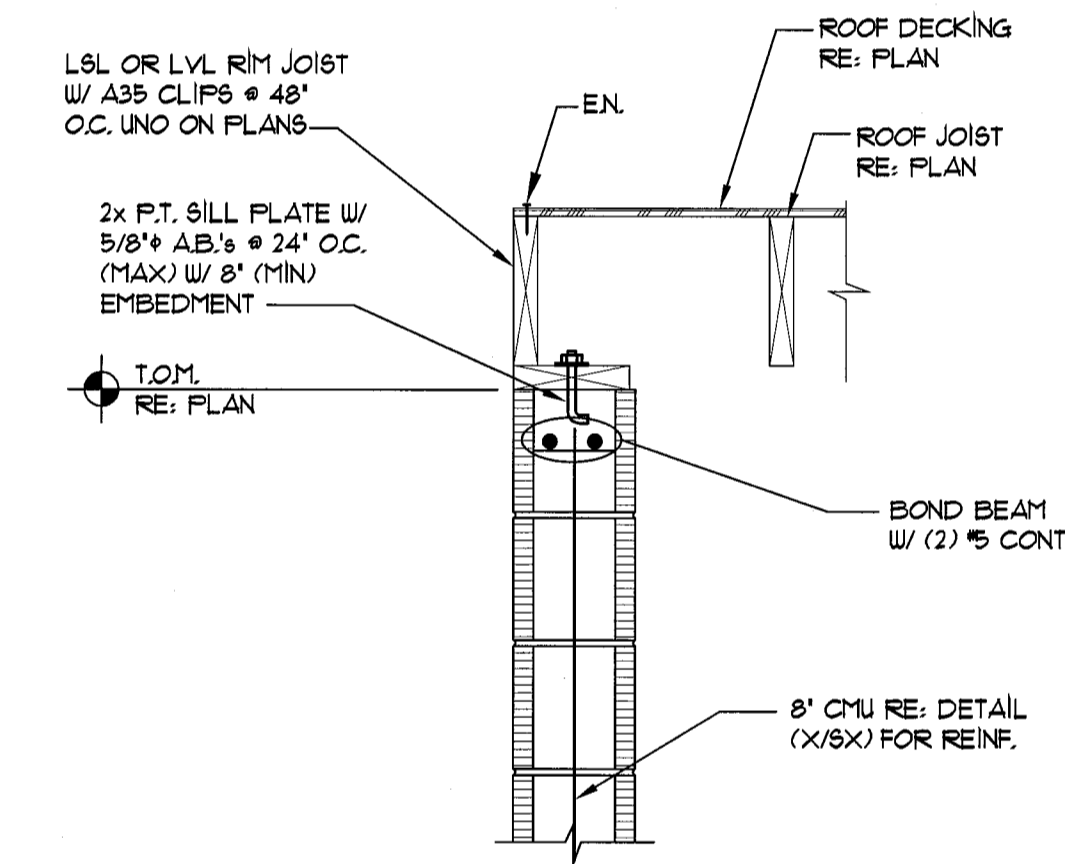


NOTE:
 INTERRUPT ALL JOINT REINF. AT CONTROL JOINTS. CONTINUE BOND BEAM REINF. THROUGH JOINTS. RE: ARCH FOR MASONRY CONTROL JOINT LOCATIONS

6 TYP. MASONRY CONTROL JOINT
SCALE: 3/4" = 1'-0"



7 ROOF FRAMING DETAIL
SCALE: 1" = 1'-0"



8 ROOF FRAMING DETAIL
SCALE: 1" = 1'-0"

